



Developing software to evaluate the liquefaction potential by using 2D numerical modeling: Applications

AUTORES:

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1. Introduction
2. Methods to evaluate the liquefaction potential
3. Software development
4. Numerical analysis
5. Conclusions

Introduction

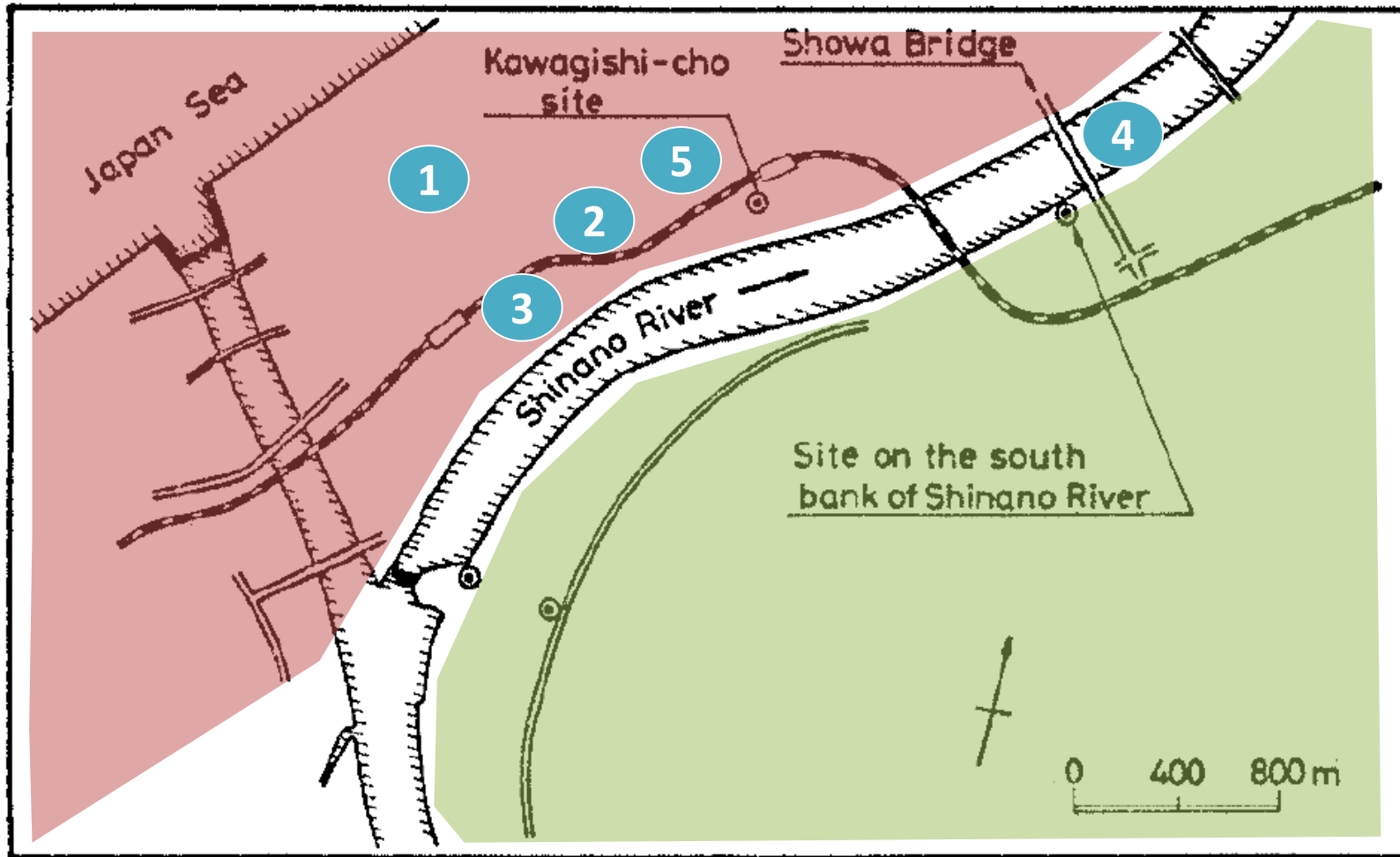


Fig. 2 Map of the city of Niigata - Japan
Source: <https://goo.gl/6yvcaQ>

Introduction



Fig. 3 Differential settlements
Source: <https://goo.gl/xYdCvb>



Fig. 4 Expulsion of water.
Source: <https://goo.gl/fVgHs9>



Fig. 5 Sand volcanoes.
Source: <https://goo.gl/3enWfz>

Introduction



Fig. 6 Failure of Showa bridge boards.
Source: <https://goo.gl/LLvR5t>

Introduction



Fig. 7 Collapse of the apartment complex of Niigata prefecture.
Source: <https://goo.gl/zukWJh>

Introduction

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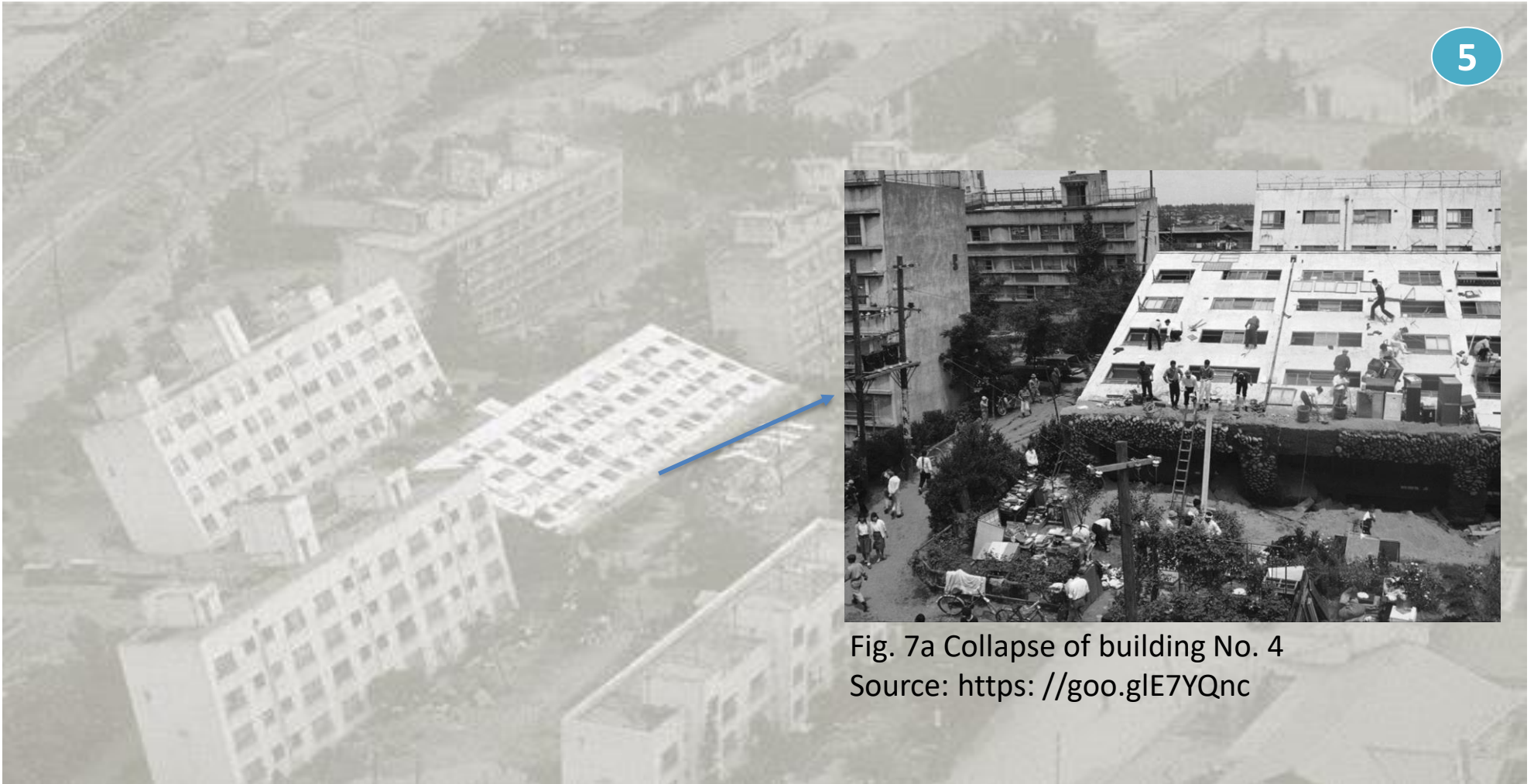


Fig. 7a Collapse of building No. 4
Source: <https://goo.gl/E7YQnc>

Fig. 7 Collapse of the apartment complex of Niigata prefecture.
Source: <https://goo.gl/zukWJh>

Introduction



Fig. 8 Earthquake of Alaska - USA (1964).
Source: <https://goo.gl/uNTxW1>



Fig. 10 Earthquake of Loma Prieta - California (1989).
Source: <https://goo.gl/phfGFU>



Fig. 9 Kobe earthquake - Japan (1995)
Source: <https://goo.gl/Xhgtif>



Fig. 11 Maule earthquake - Chile (2010).
Source: <https://goo.gl/V3JDYA>



Fig. 12 Sismo de Pedernales - Ecuador (2016).
Source: EERI

Introduction



a)



d)



e)

Fig. 13 Port of Manta, Pedernales earthquake - Ecuador (2016).
Source: EERI

Fig. 14 Calceta, Pedernales Earthquake - Ecuador (2016).
Source: EERI



Introduction



Fig. 15 Manta shrimp, Pedernales earthquake - Ecuador (2016).
Source: EERI



Fig. 16 Manta, Pedernales Earthquake - Ecuador (2016).
Source: EERI

Introduction



Fig. 17 Sand volcanoes



Fig. 18 Manta, Mass movements

Introduction

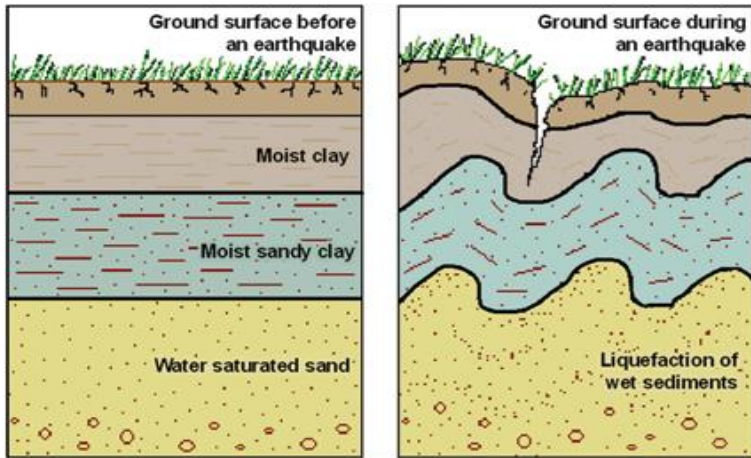


Fig. 19 Deformation of the soil due to liquefaction
Source: <https://goo.gl/GGJQRp>

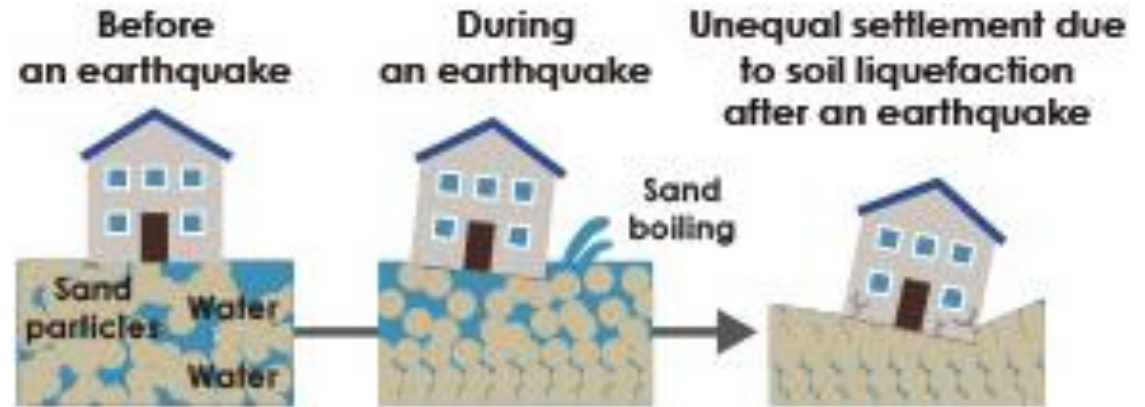


Fig. 20 Representation of the soil during liquefaction.
Source: <https://goo.gl/Mz6TJt>

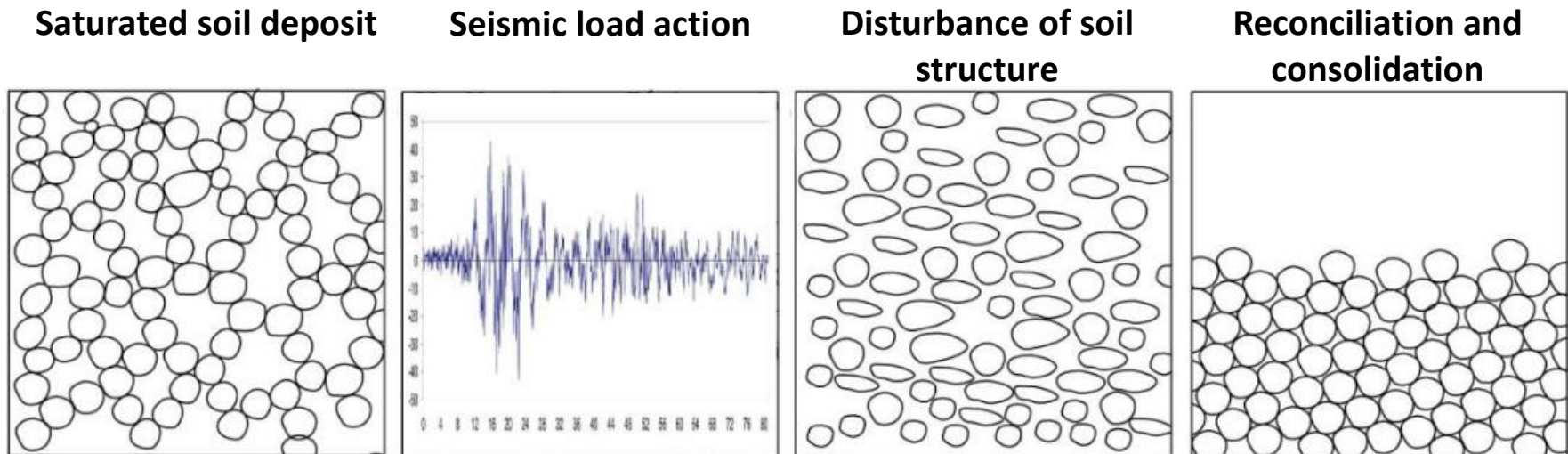


Fig. 21 Proceso de generación del fenómeno de licuefacción.
Fuente: <https://goo.gl/emGHJU>

Methods to evaluate the liquefaction potential

□ Empirical Methods



Fig. 22 Field observations.
Source: <https://goo.gl/FHbD9Y>

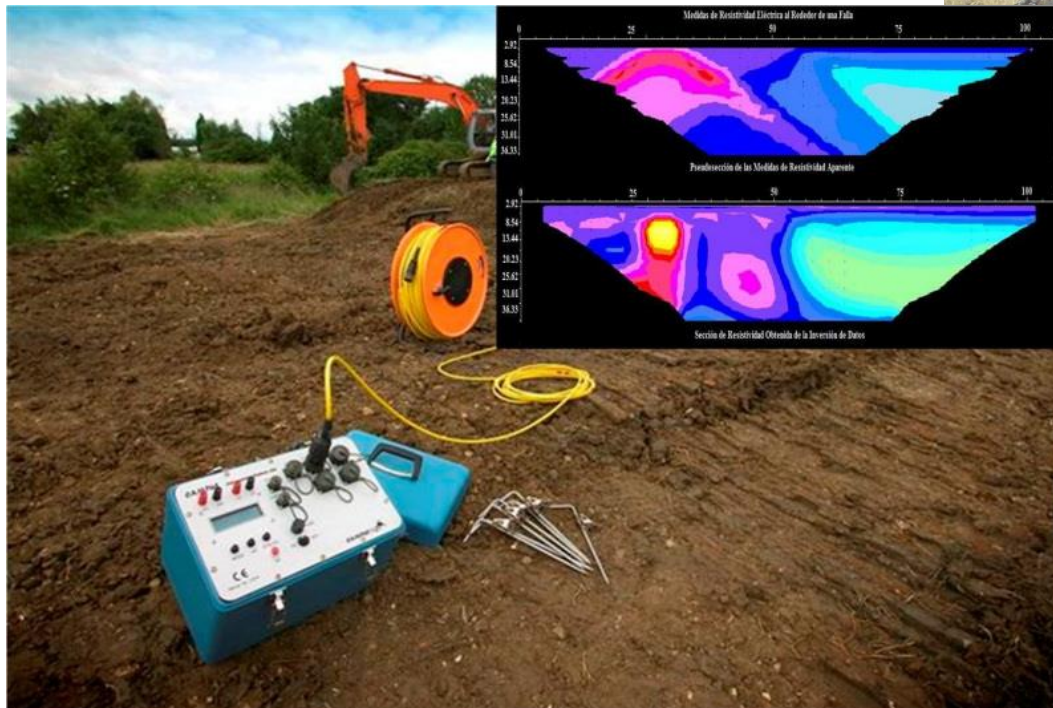


Fig. 23 Geophysical prospecting tests.
Source: <https://goo.gl/jgYx9c>

Methods to evaluate the liquefaction potential

□ Semi-empirical methods

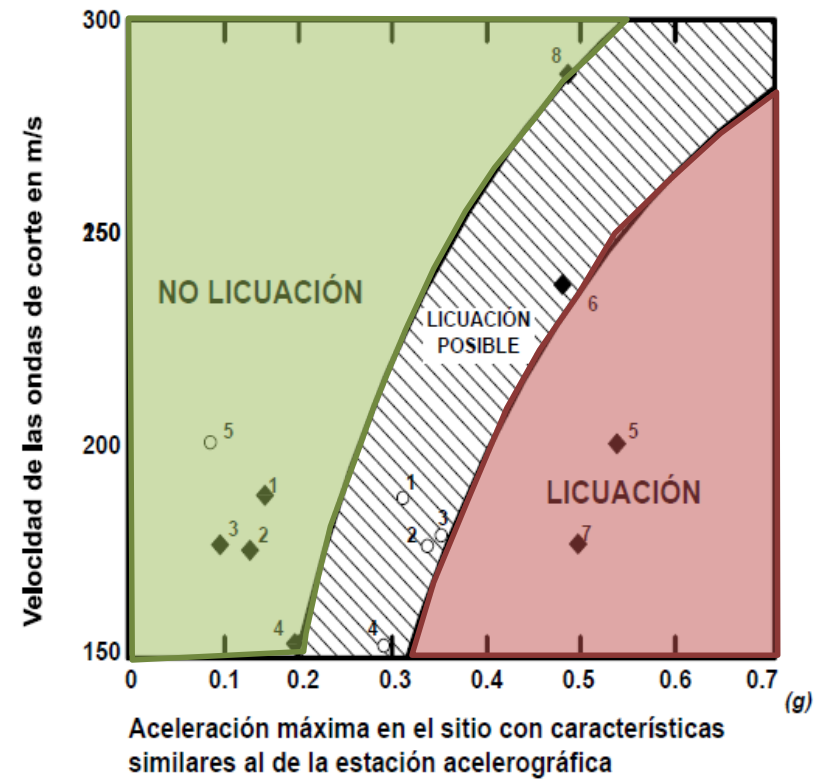
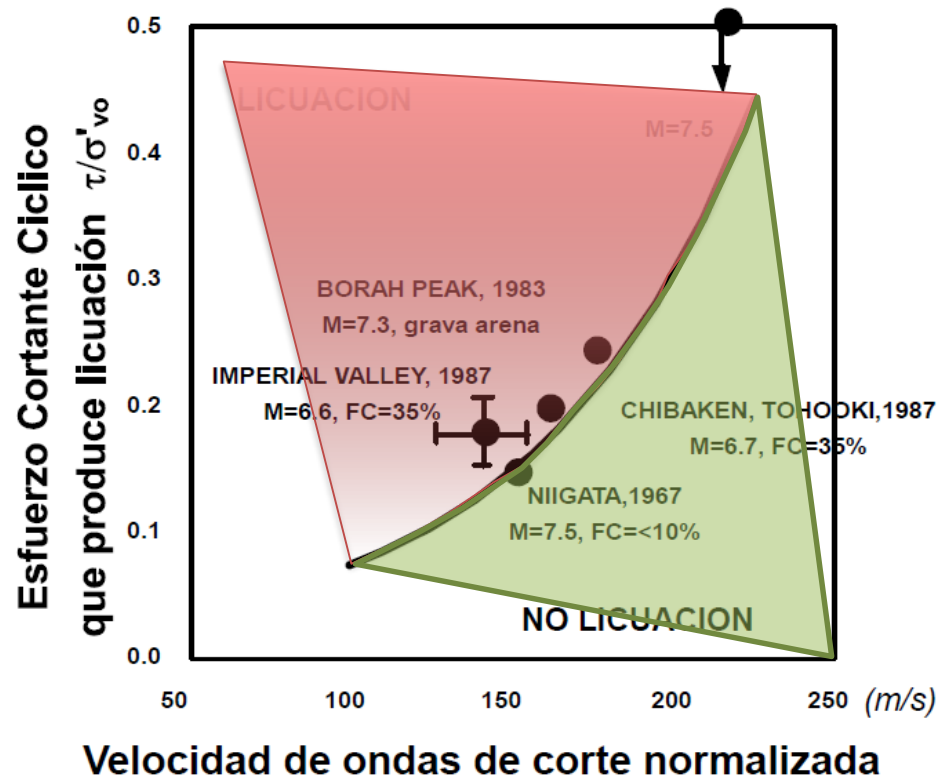


Fig. 24 Correlation A of the simplified method
Source: García Nuñez, 2007

Fig. 25 Correlation B of the simplified method
Source: García Nuñez, 2007

Methods to evaluate the liquefaction potential

Numerical methods

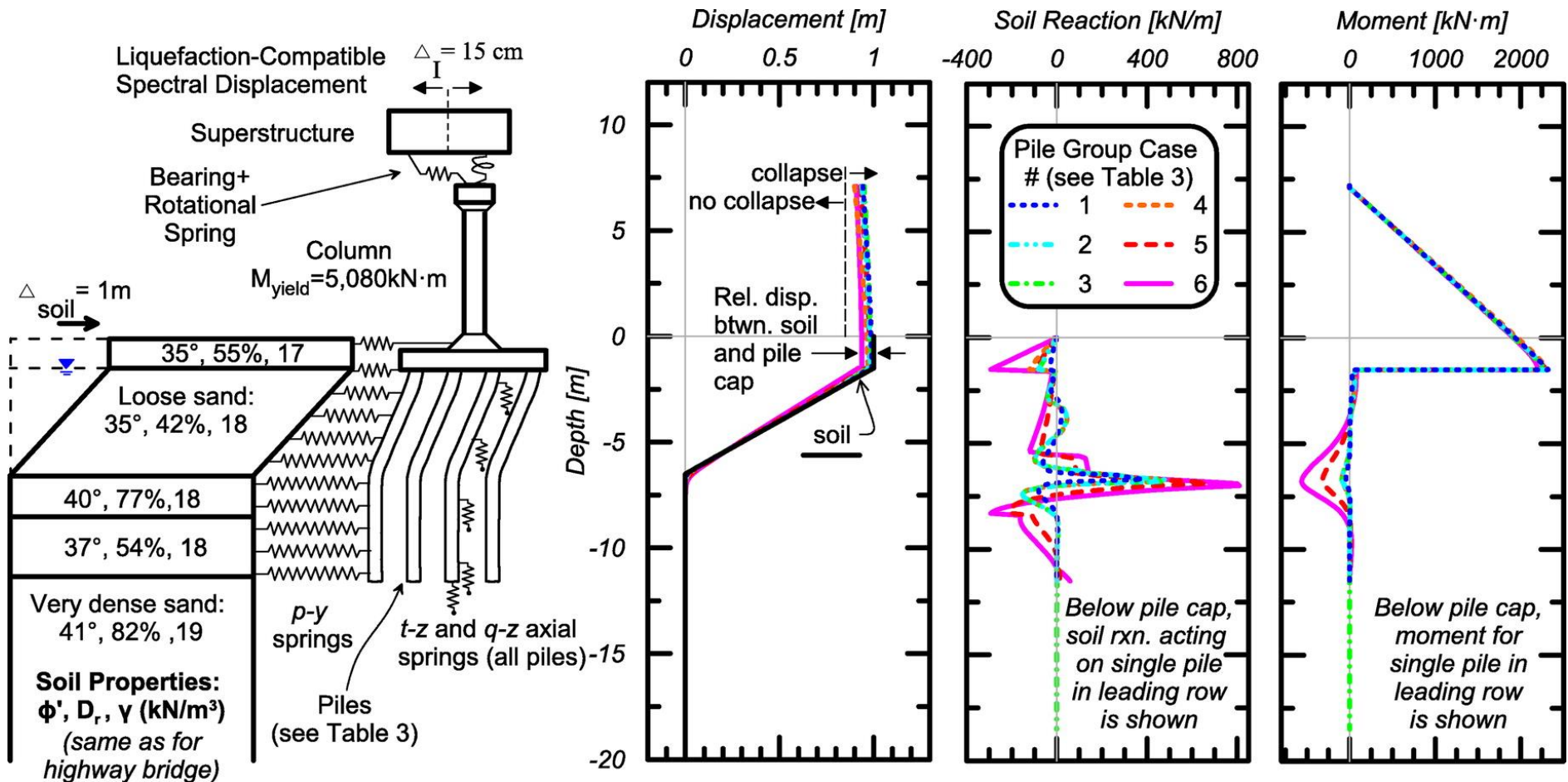
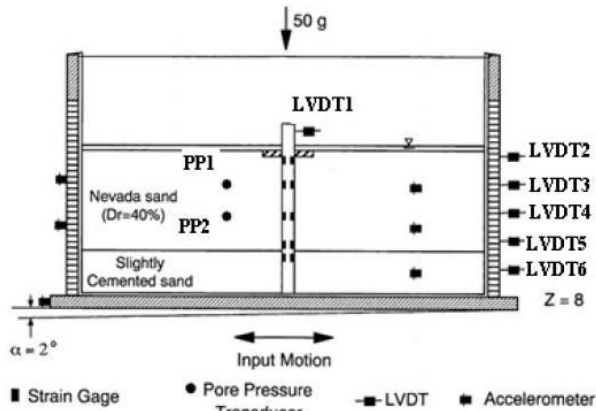


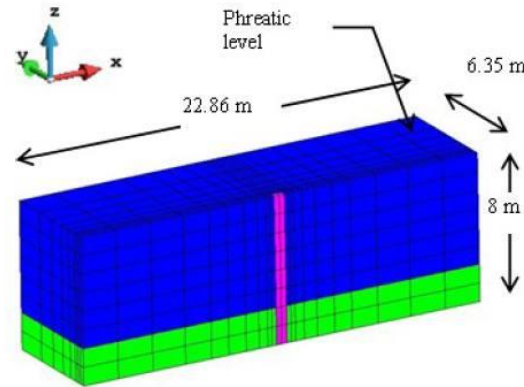
Fig. 26 Numerical representation of a bridge pile affected by liquefaction by finite elements of a physical model.
Source: <https://goo.gl/H9gKj4>

Methods to evaluate the liquefaction potential

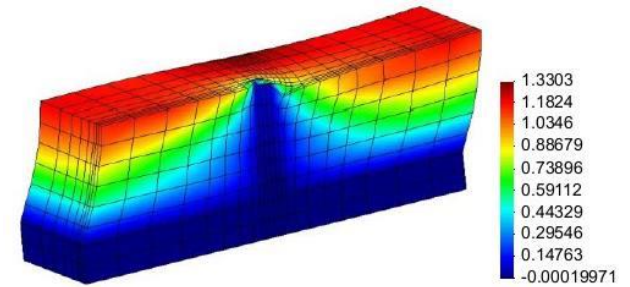
Numerical methods



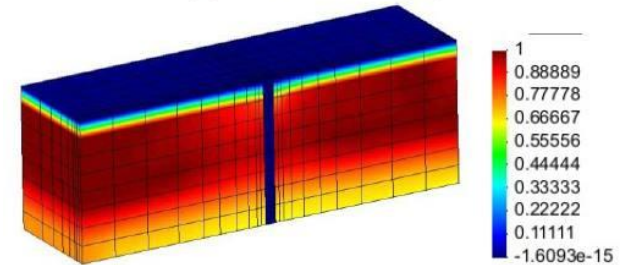
(a) Instrumentation used in model 3



(b) Finite element mesh (in prototype scale)



(a) Deformed mesh (m)



(b) Potential of liquefaction (r_u)

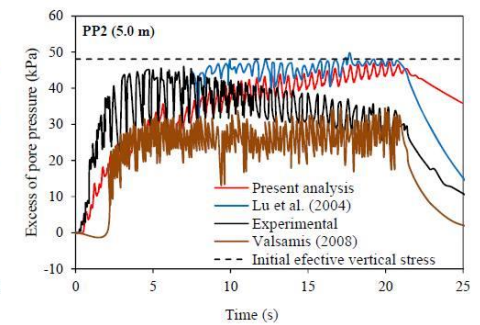
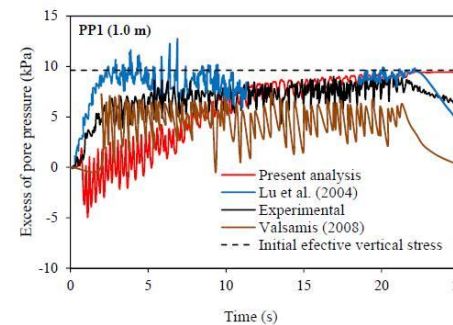
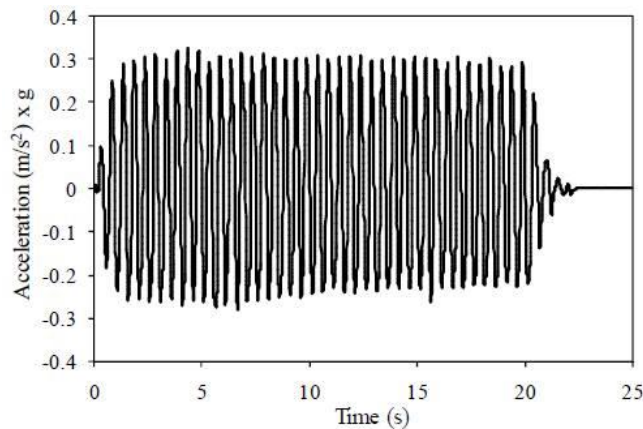


Fig. 27 Soil-Pile System Under Harmonic Load (Abdoun, 1997)

Methods to evaluate the liquefaction potential

Numerical methods

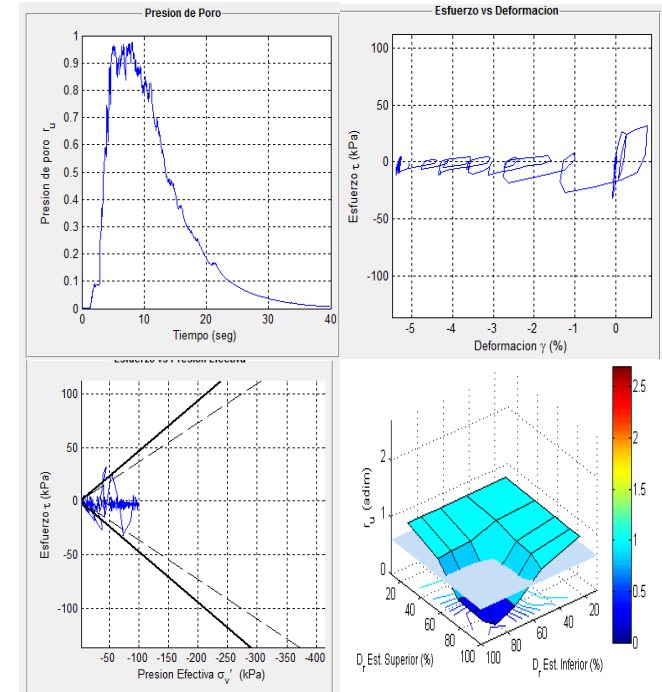
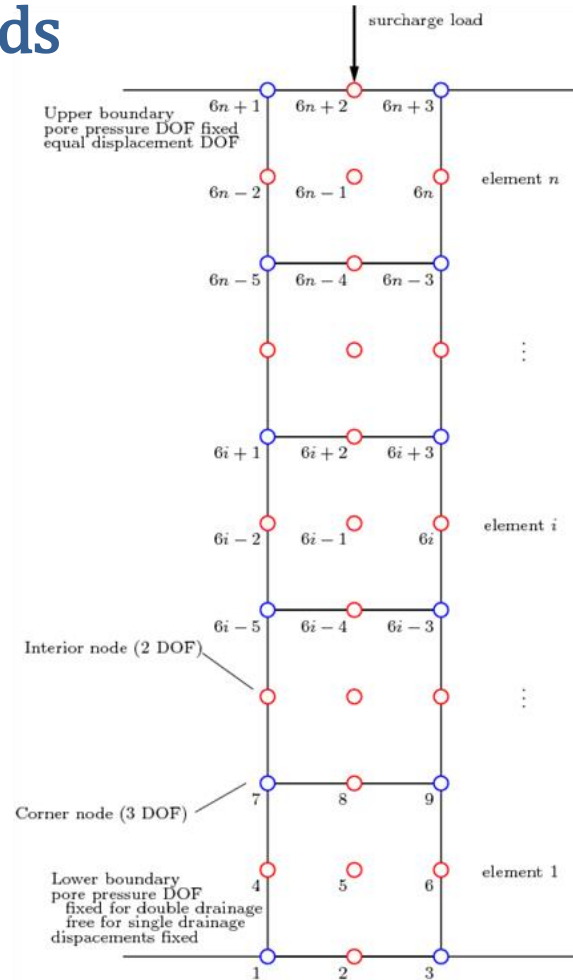
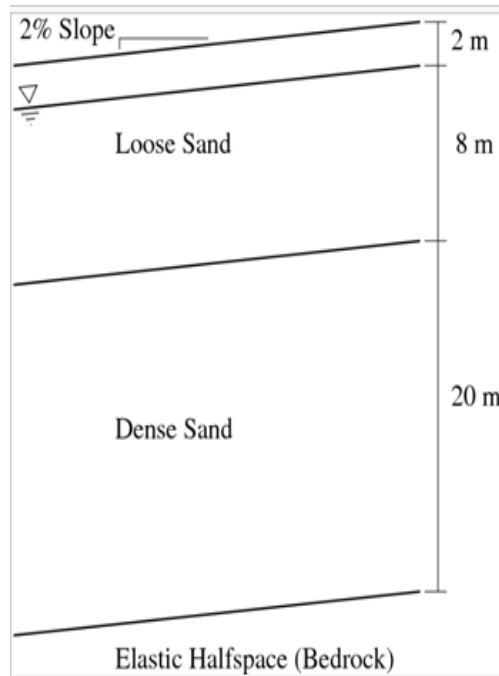
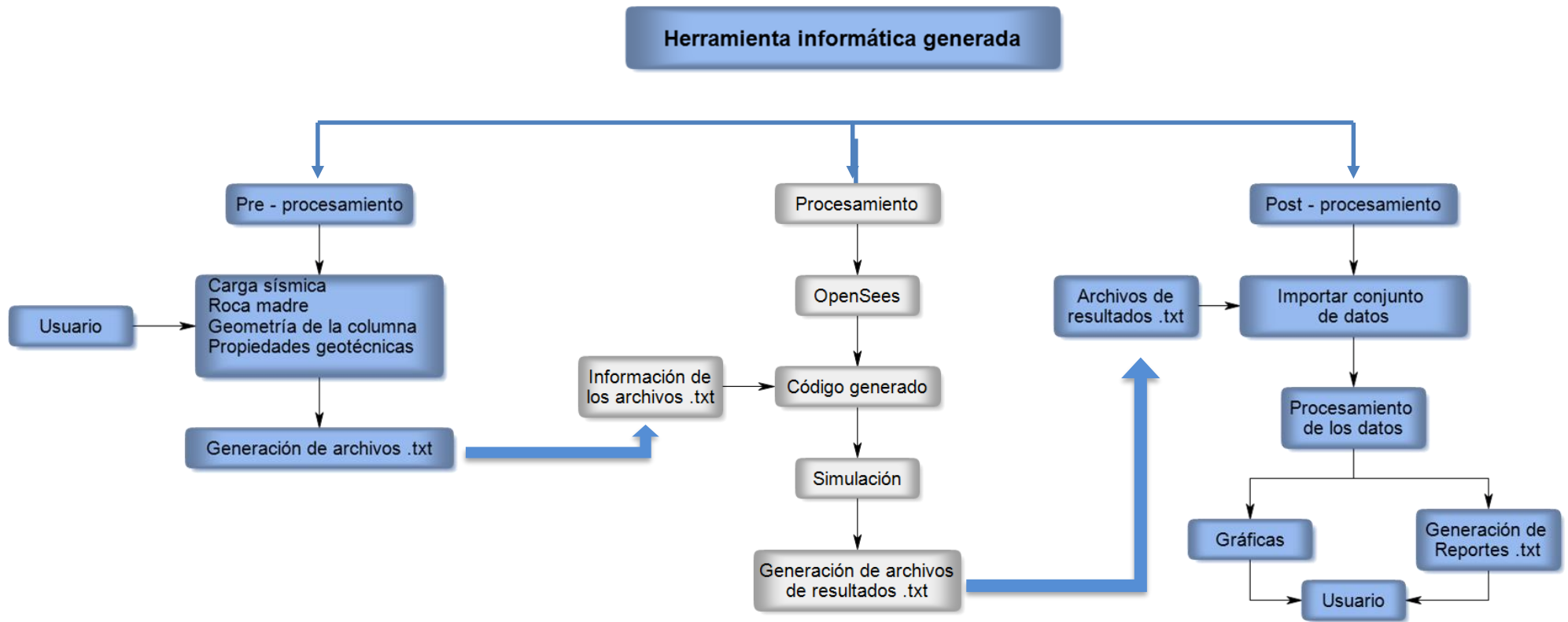


Fig. 28 Representation by finite elements of a physical model.
Source: <https://goo.gl/H9gKj4>

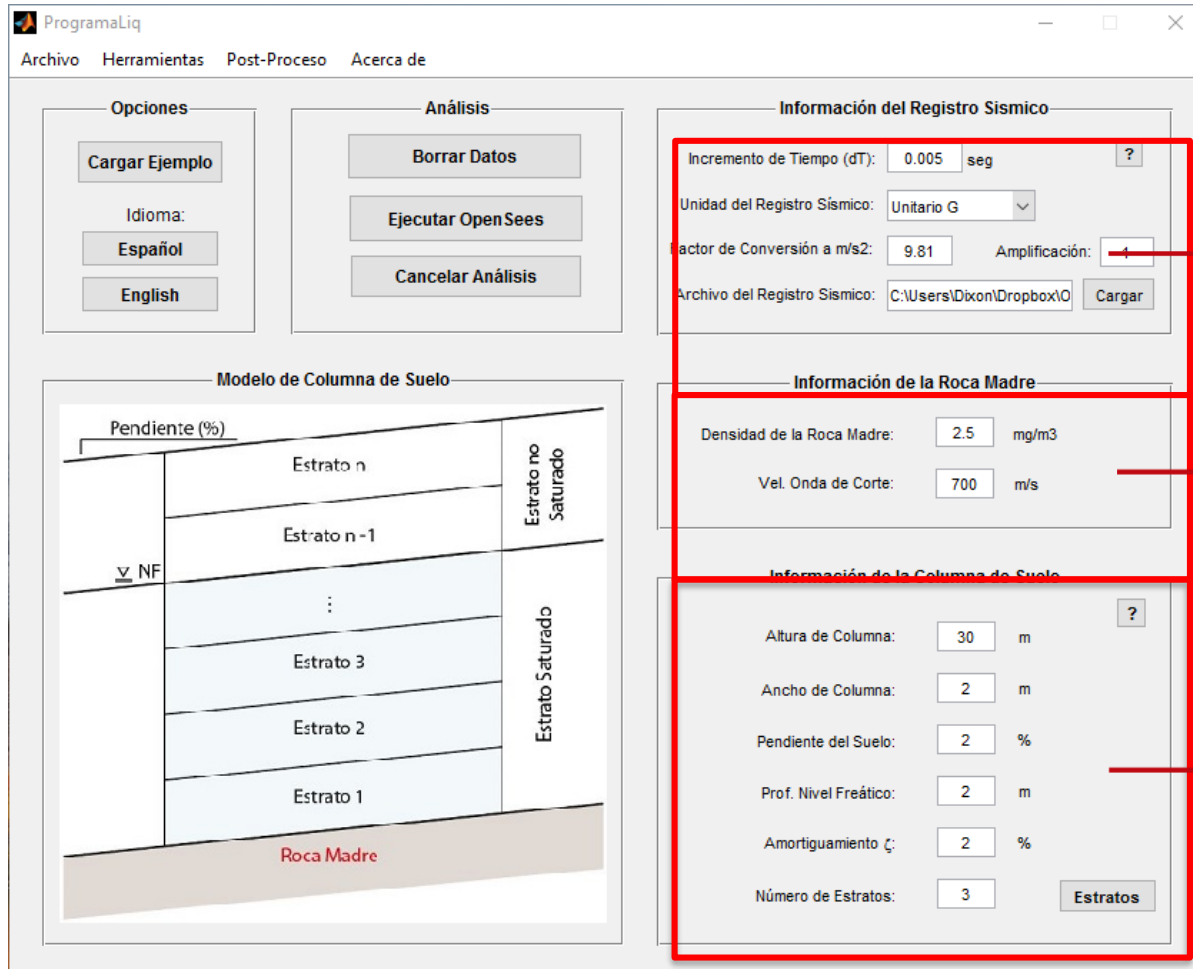
Software development

□ Flowchart

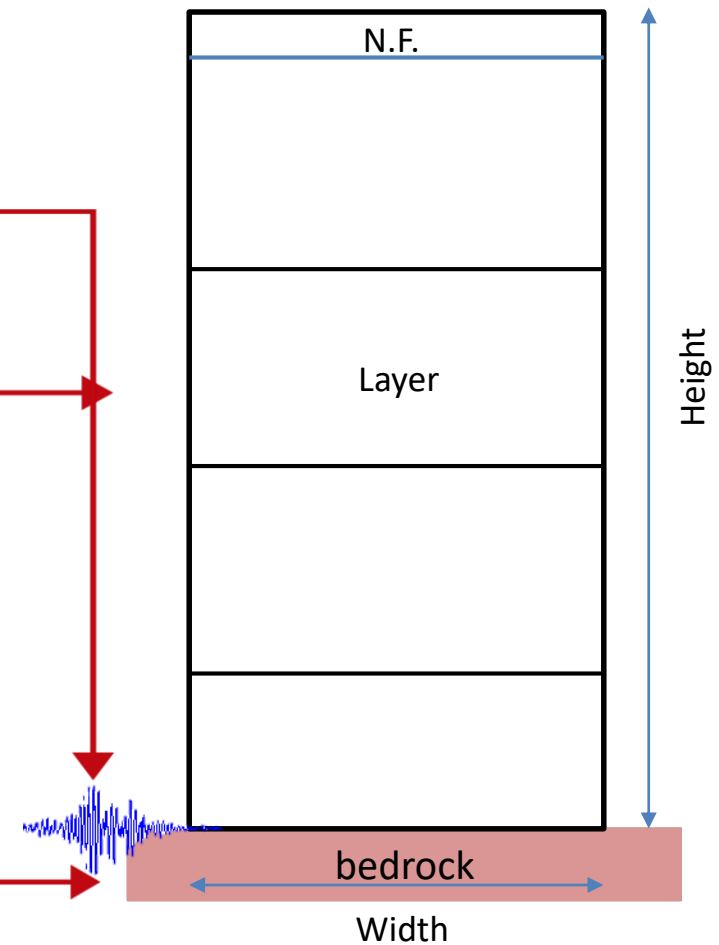


Pre-process

Main interface

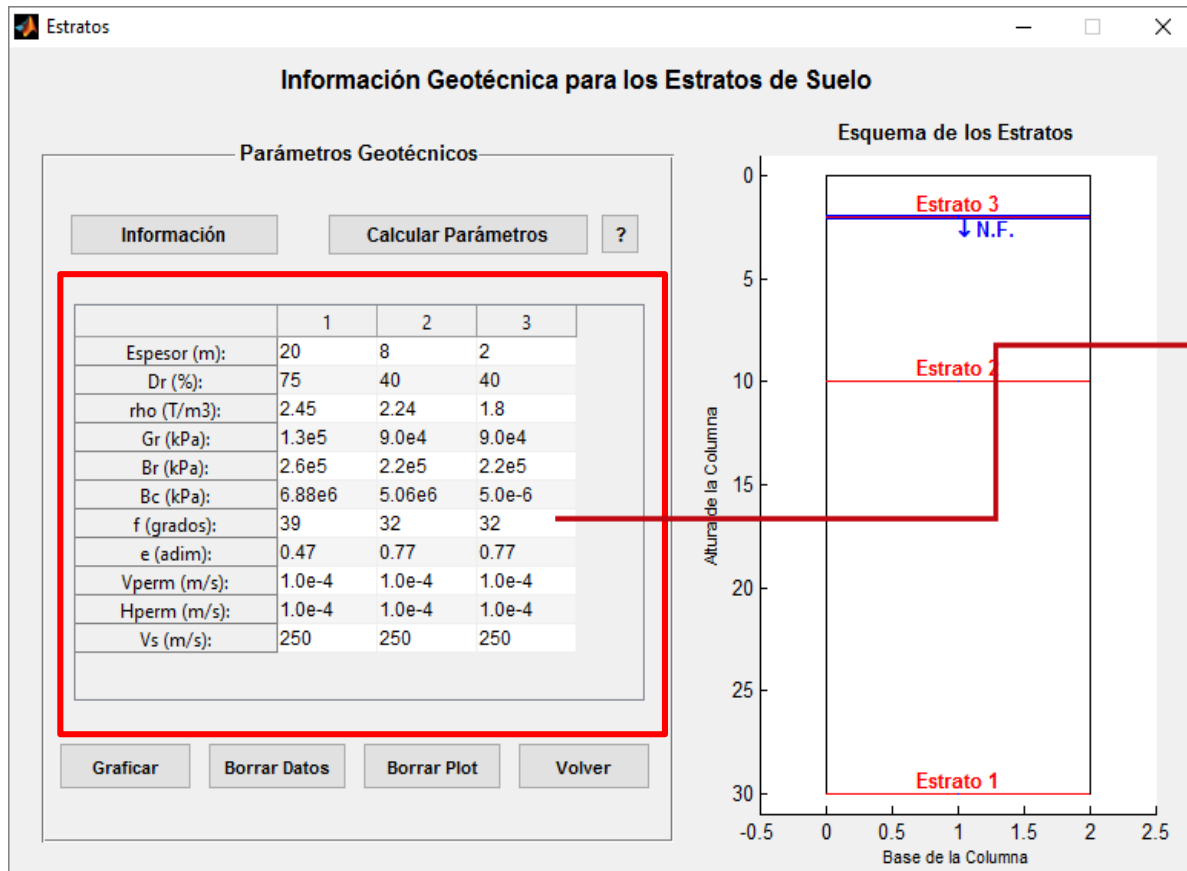


Definition of a soil column

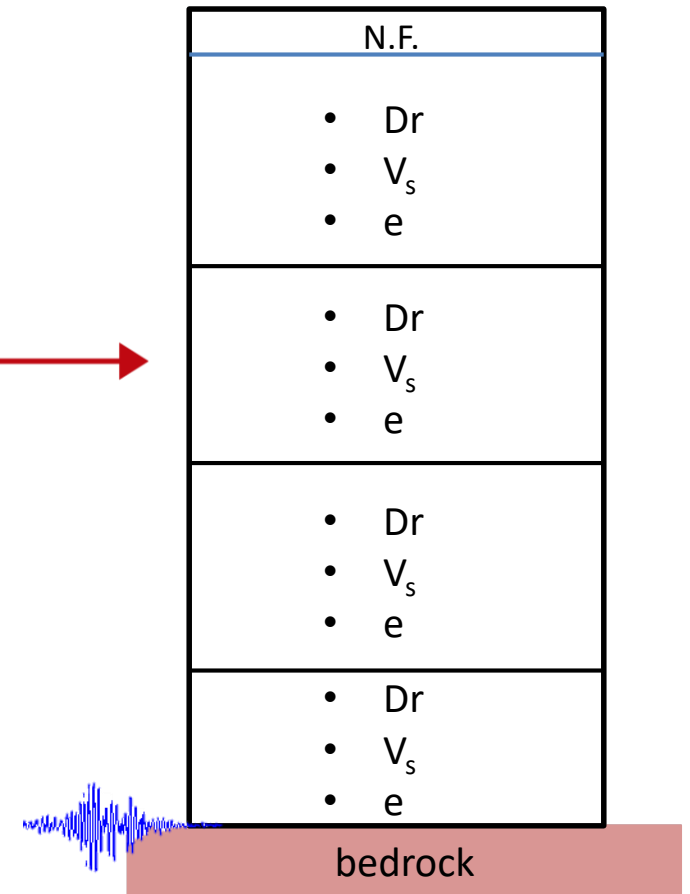


Pre-process

Auxiliary interface

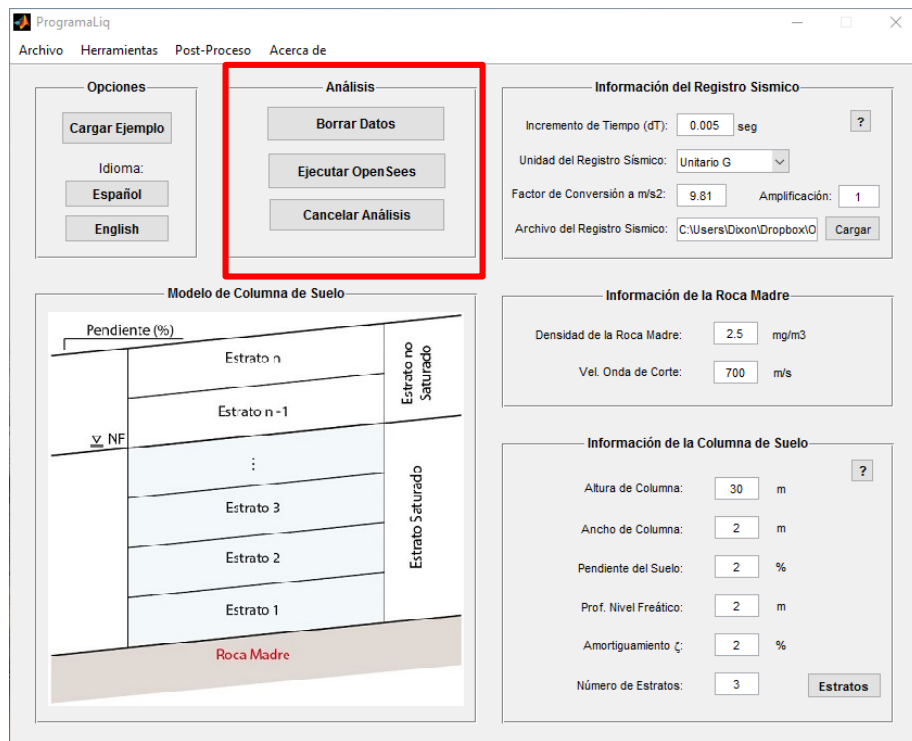


Definition of a soil column

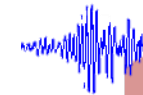
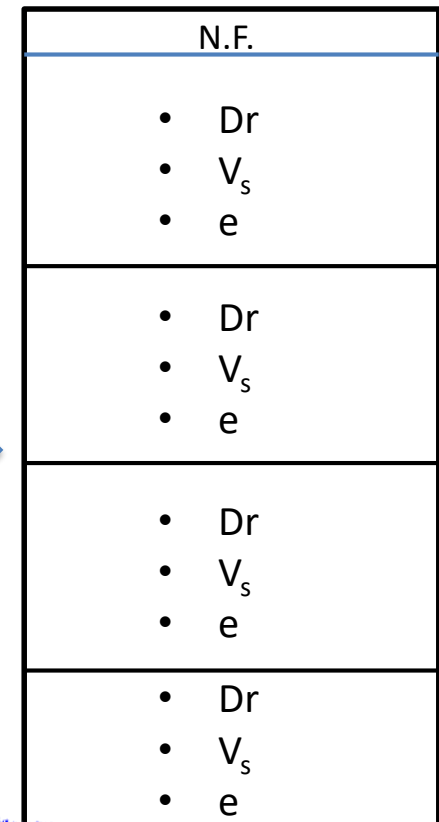


□ Analysis process

Main interface



Soil column model



bedrock

□ Description of the model

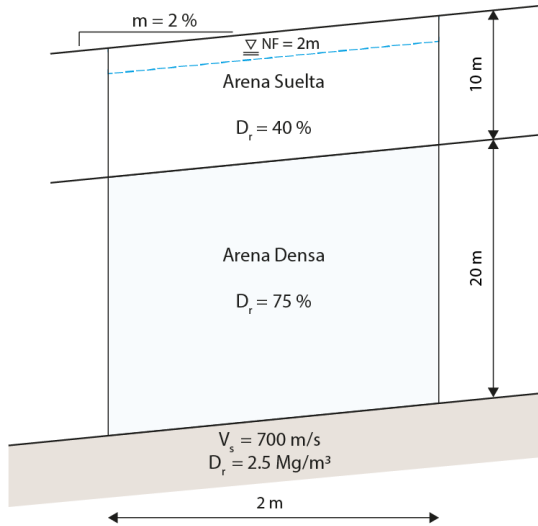


Fig. 29 Stratigraphy

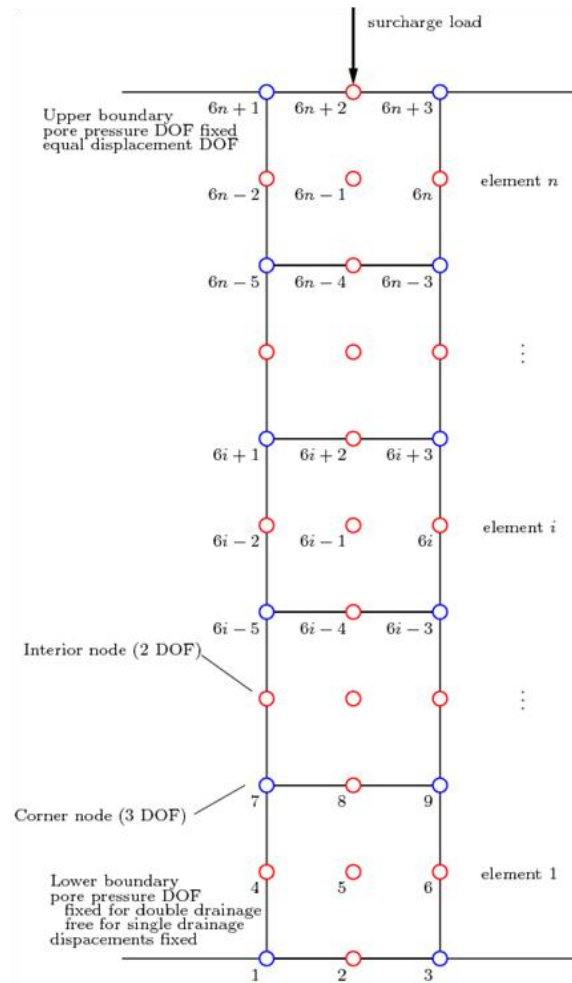
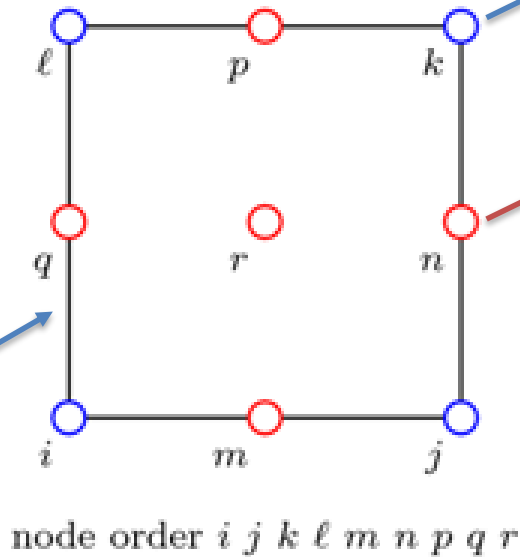
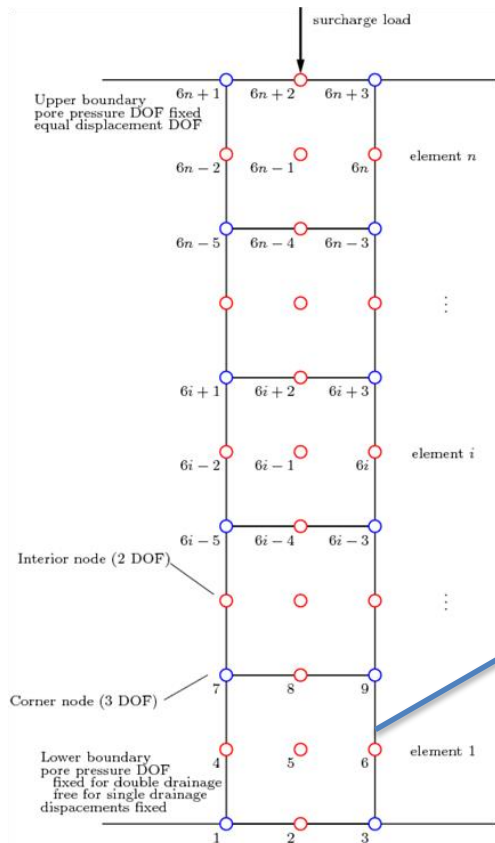


Fig. 30 Generation of a model soil column in OpenSees.

- Definition of meshing
- Geometry of the column
- Support Bedrock
- Definition of the elements
 - Outside nodes
 - Interior nodes
 - Element material
- Static analysis
- Dynamic analysis

Example prepared by: [Christopher McGann](#) and [Pedro Arduino](#), University of Washington

□ Meshing



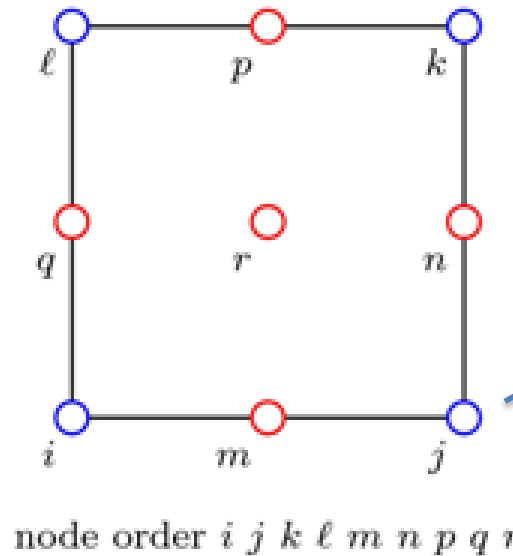
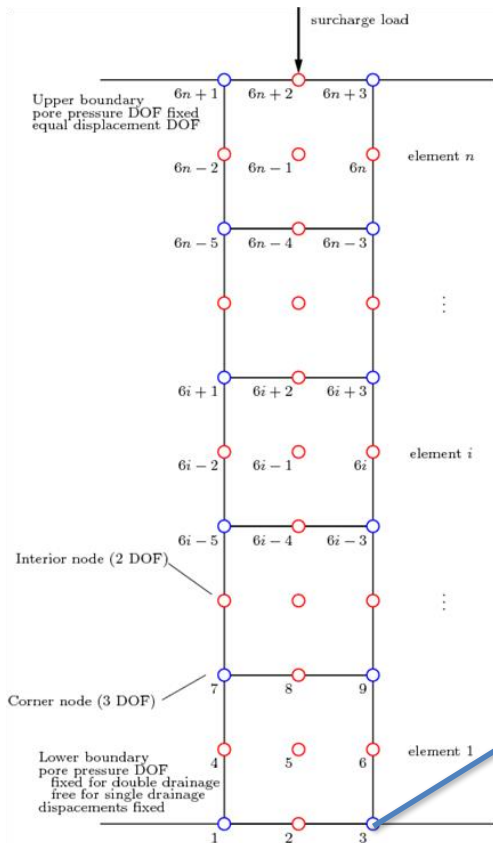
3 DOF
2 Translational
1 Pore pressure

2 DOF
2 Translational

9_4_QuadUP

Model soil column in OpenSees.

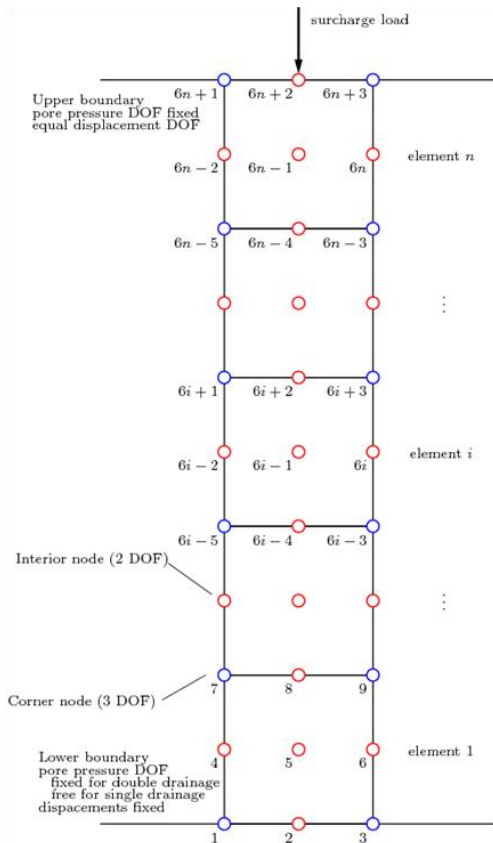
□ Boundary conditions



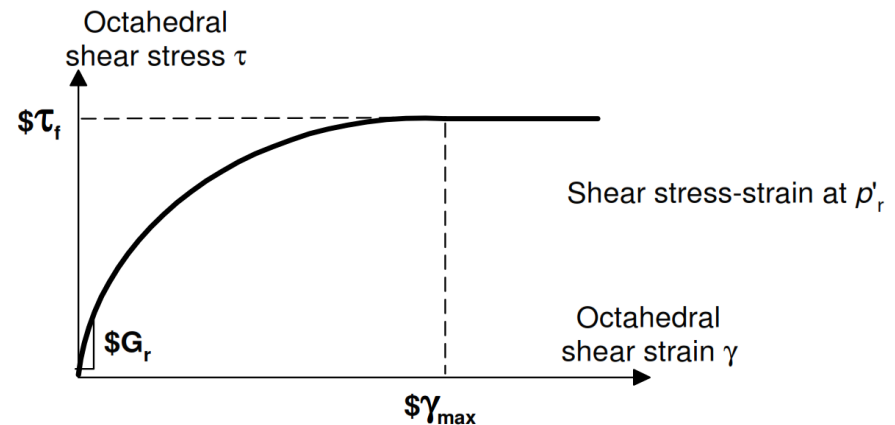
- fixed against vertical translation
- fixed in the pore pressure pore in pressure nodes above the groundwater

Model soil column in OpenSees.

Material and Element Definitions

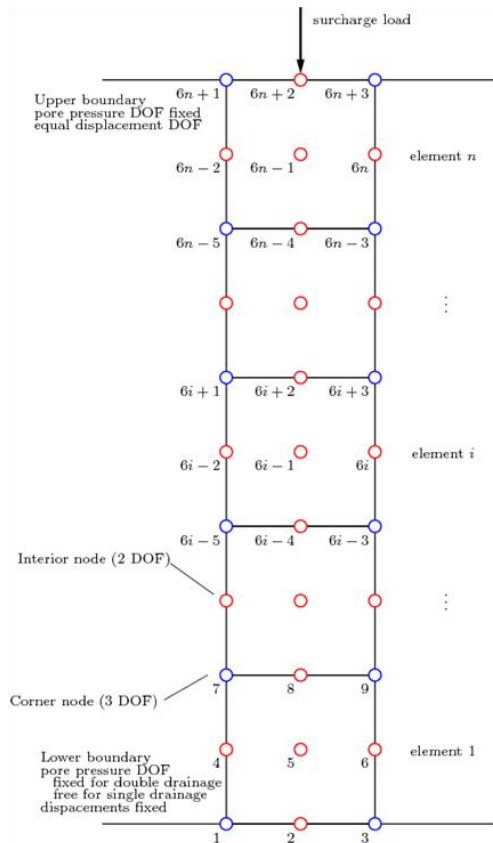


```
nDMaterial PressureDependMultiYield02 $tag $nd $rho $refShearModul
$refBulkModul $frictionAng $peakShearStra $refPress
$pressDependCoe $PTAng $contract1 $contract3 $dilat1 $dilat3
<$noYieldSurf=20 <$r1 $Gs1 ...> $contract2=5. $dilat2=3.
$liquefac1=1. $liquefac2=0. $e=0.6 $cs1=0.9 $cs2=0.02 $cs3=0.7
$pa=101>
```



Model soil column in OpenSees.

Recorders and Analysis



Analysis Gravitation



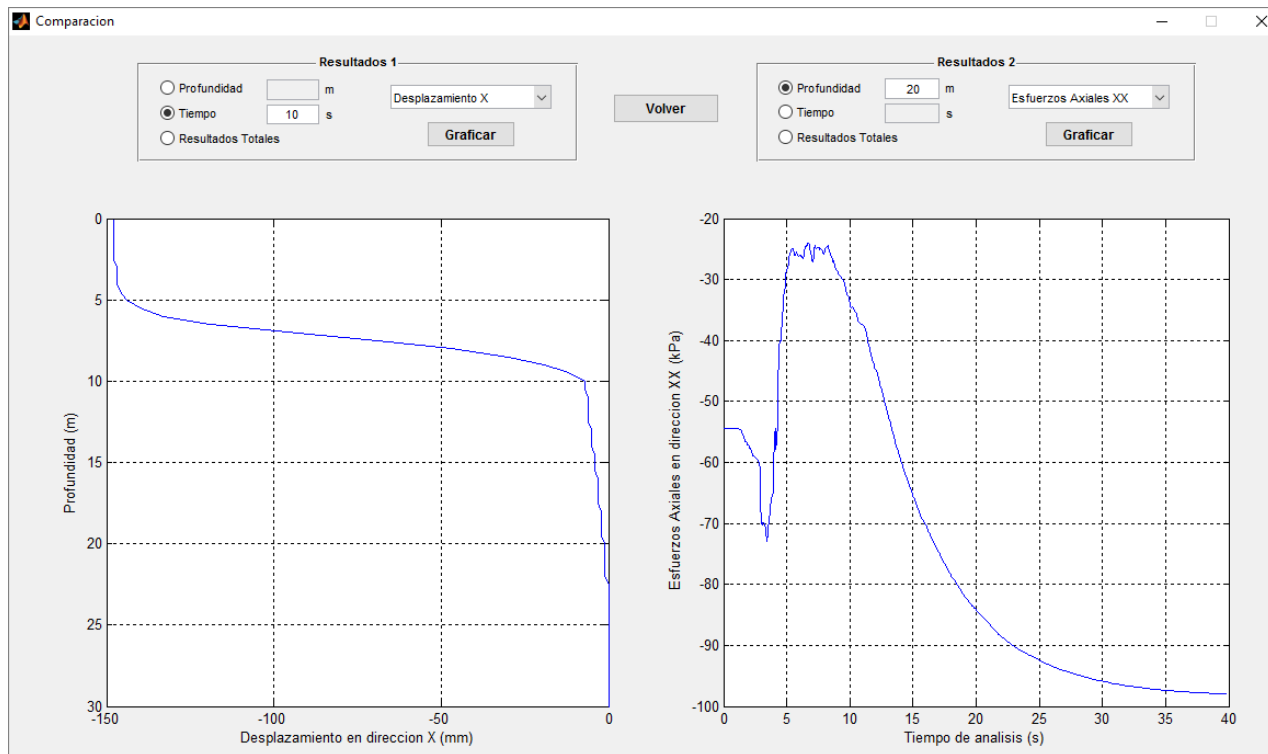
Analysis Dynamic

- Acceleration X y Y
- Displacement X y Y
- PorePressure
- Strain

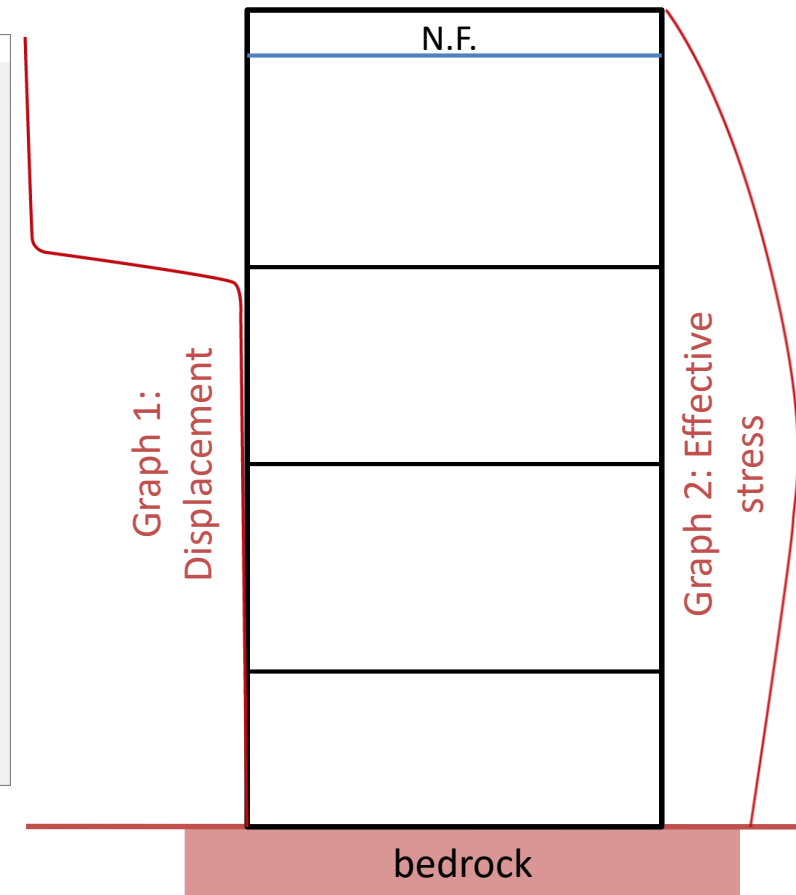
Model soil column in OpenSees.

Post-process

Comparison



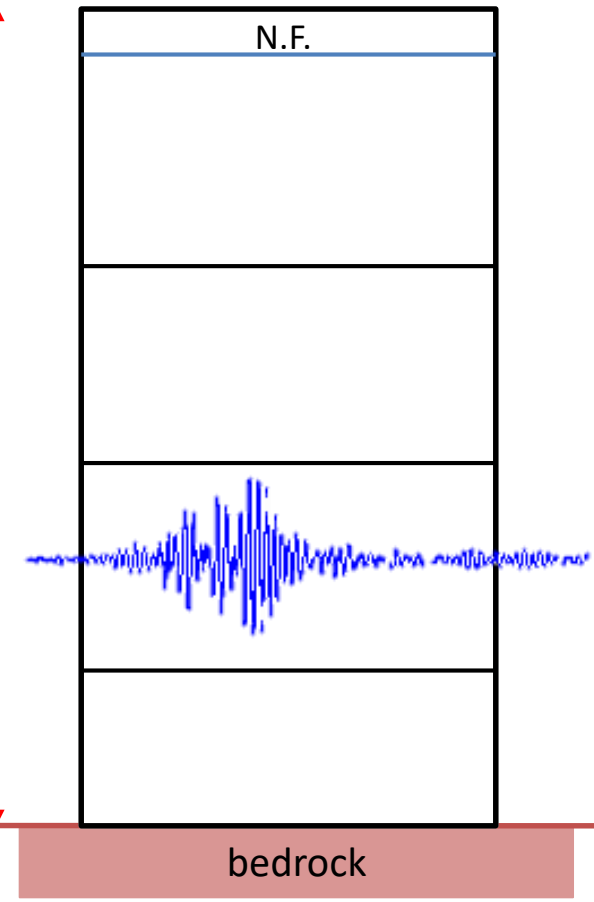
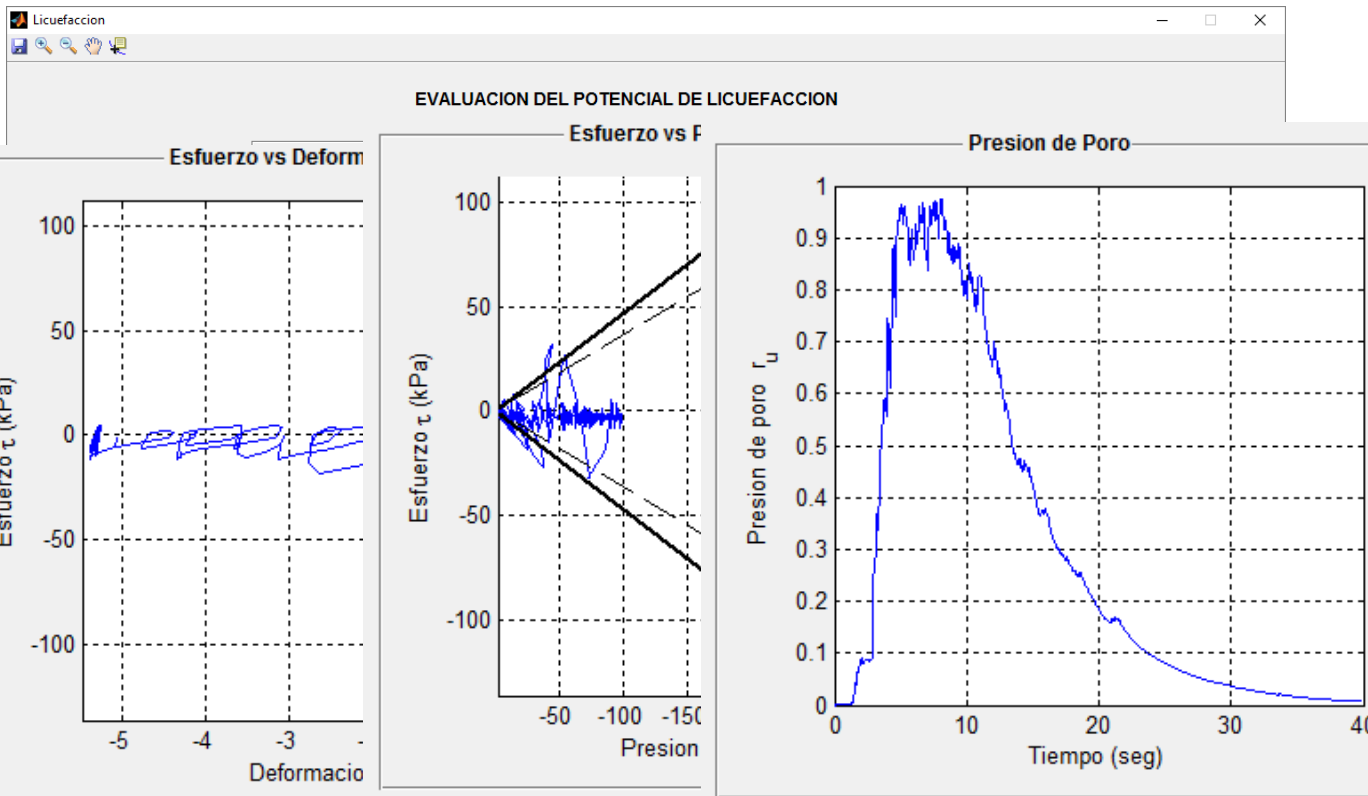
Soil column model



Post-process

Comparison

Soil column model



Stress vs Deformation Stress vs. Effective Pressure Excess pore pressure

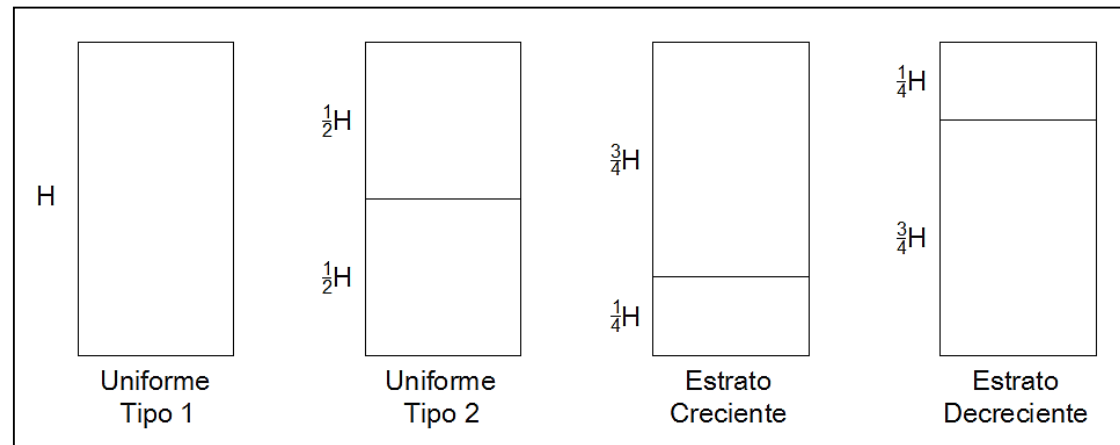
Numerical modeling

Numerical modeling

□ Definición de escenarios para análisis numérico

- Height: 15m 30m 60m

- Layers:



- Water table: $0H$ $\frac{1}{4}H$ $\frac{1}{2}H$ $1H$

- Seismic load: 0,065 g 0,25 g 0,50g 0,95 g 1,41g

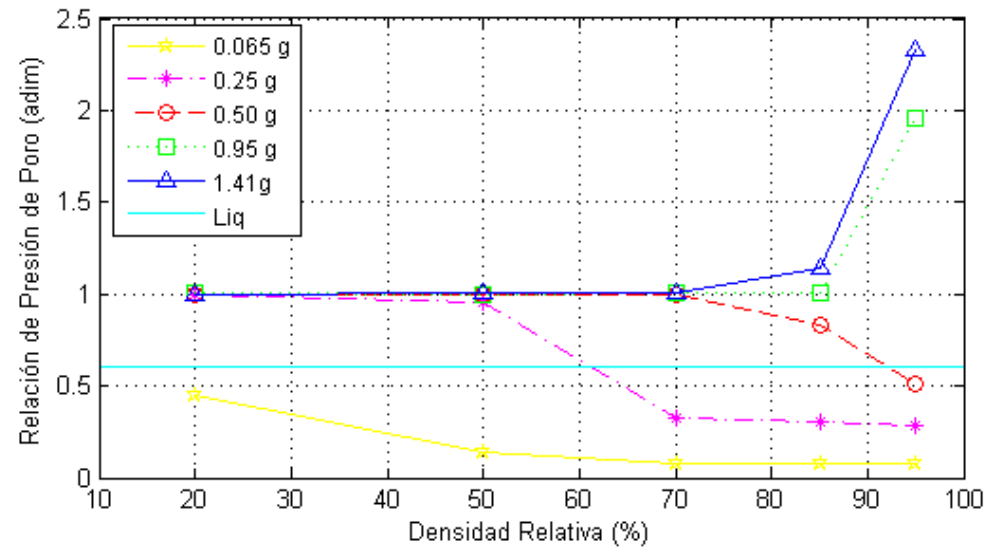
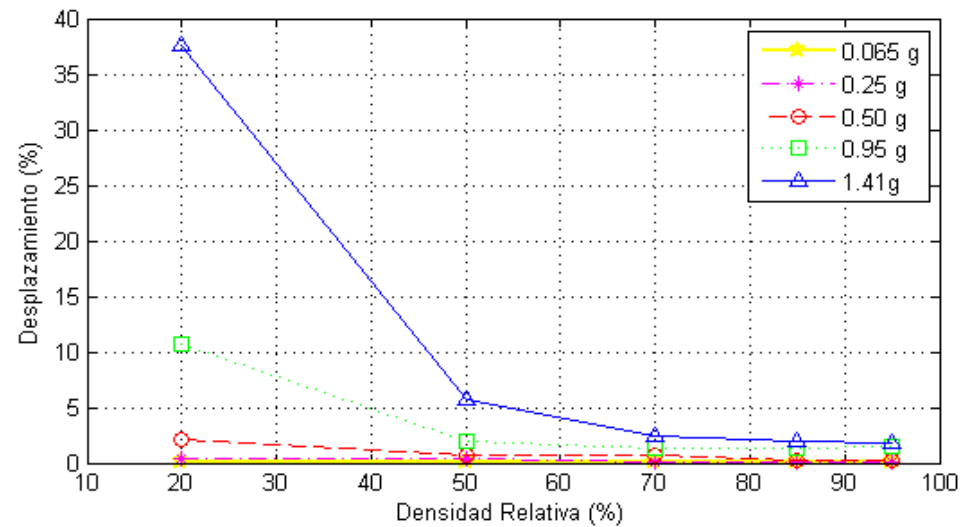
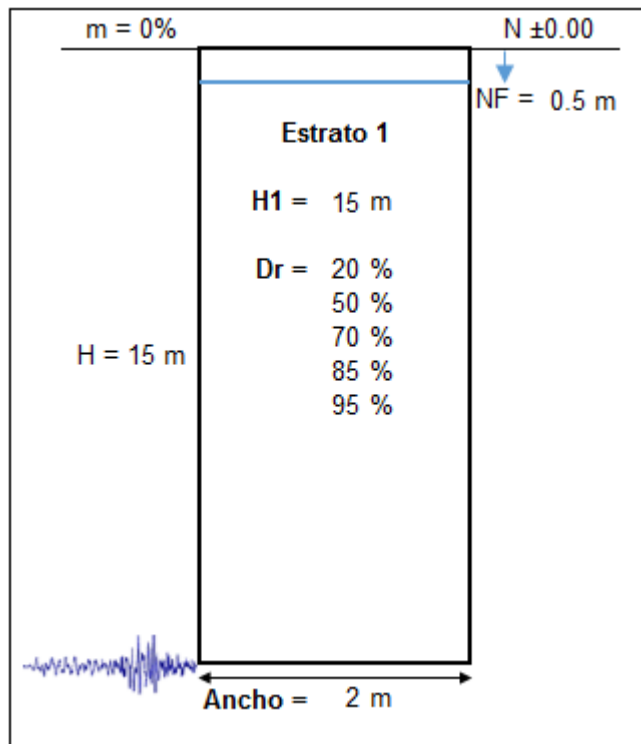
- Width: 2m

- Slope: 0%

Numerical modeling

- Results of the numerical models for the analysis of soil liquefaction.

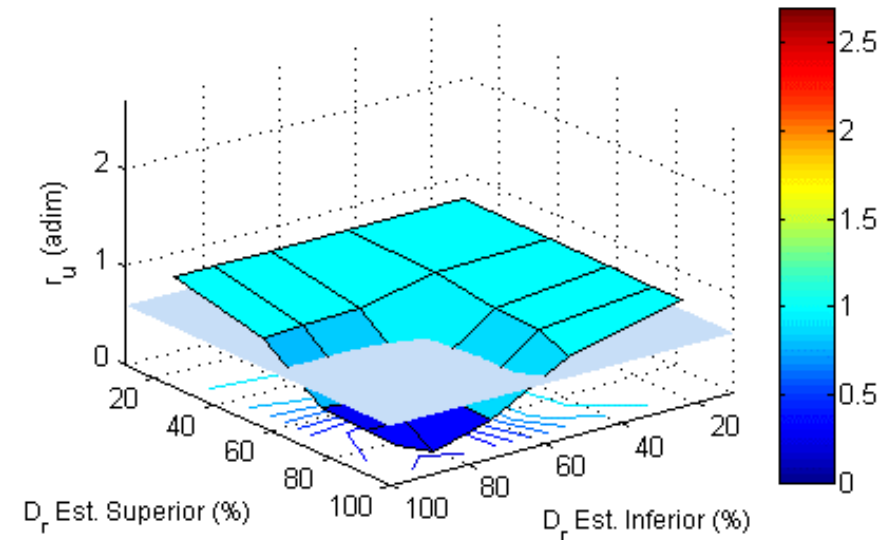
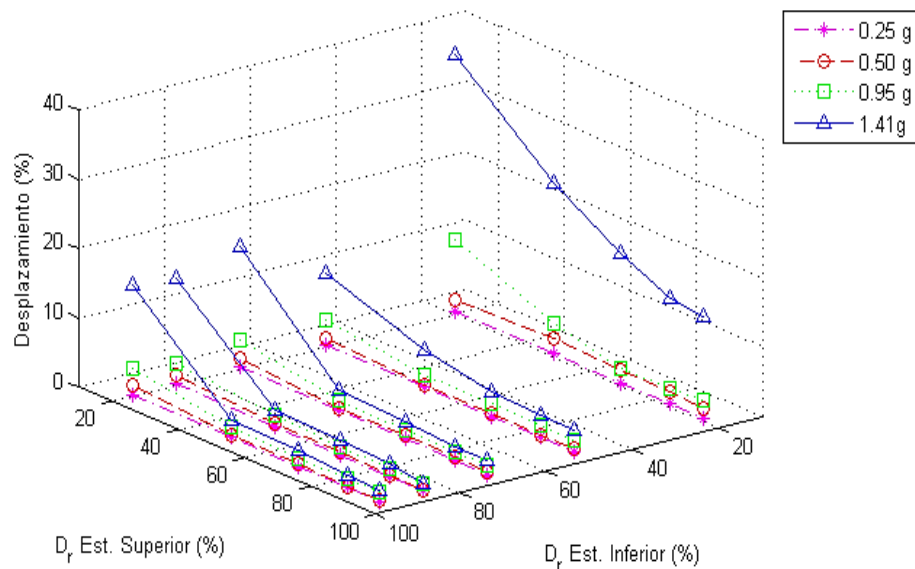
Scenario 1



Numerical modeling

Results of the numerical models for the analysis of soil liquefaction.

Representation of graphs from scenario 2



Graphs of displacement in function of the height of the column of floor

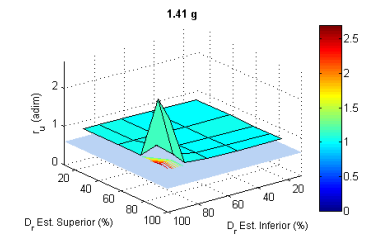
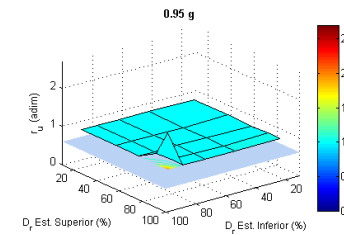
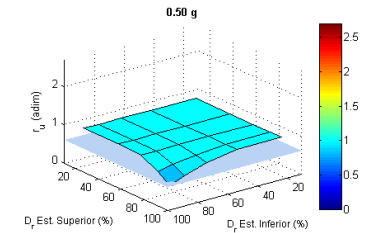
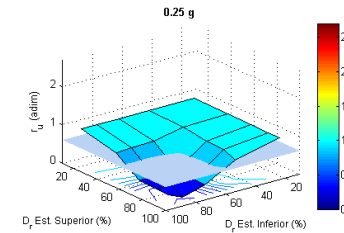
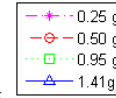
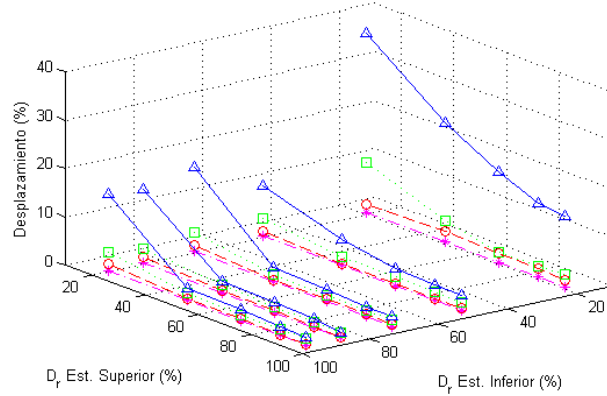
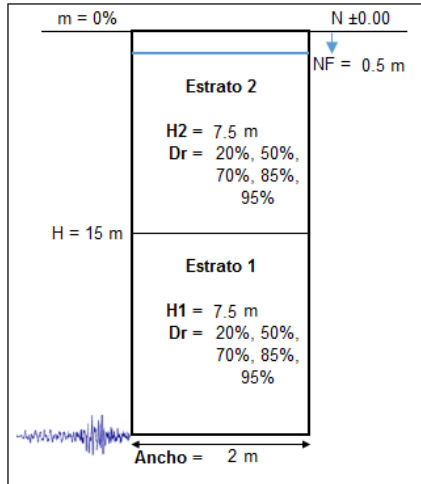
Pore pressure graphs



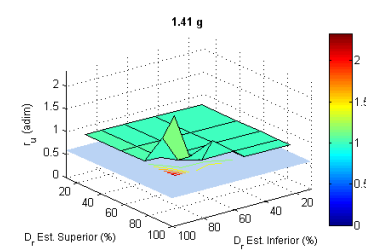
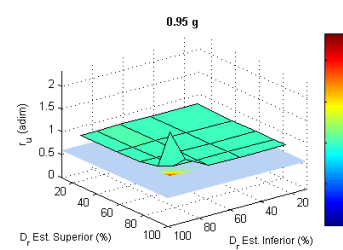
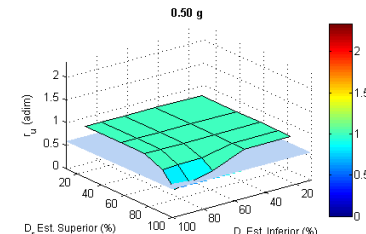
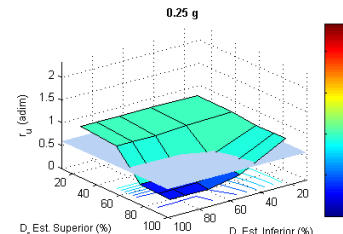
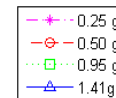
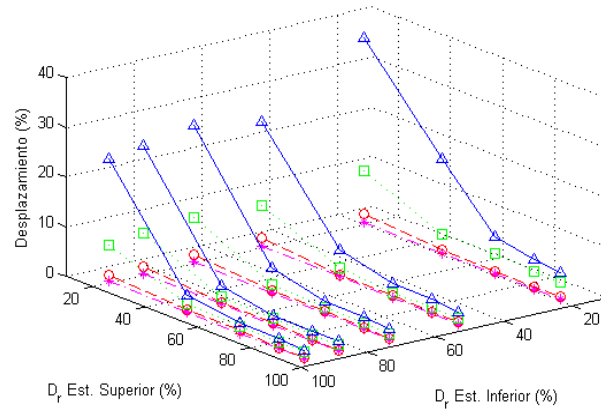
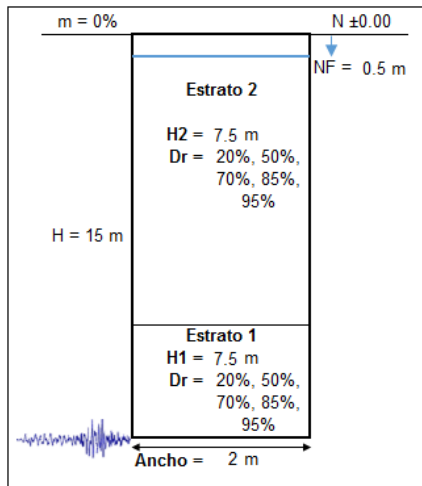
Influence of Soil Stratification

Numerical modeling

Scenario 2

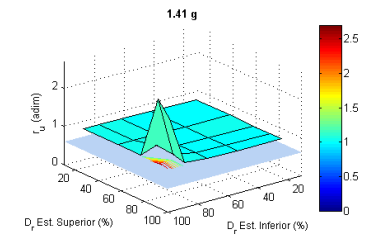
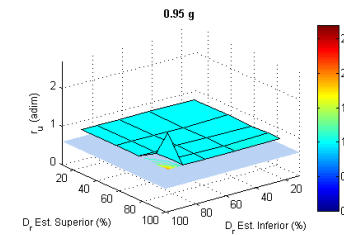
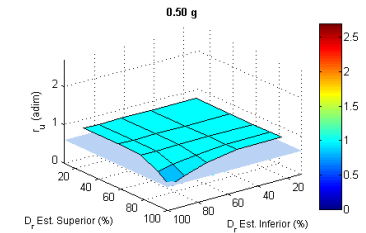
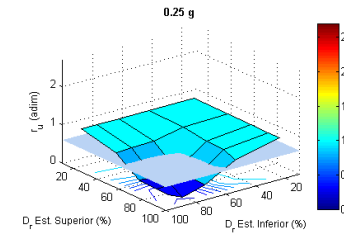
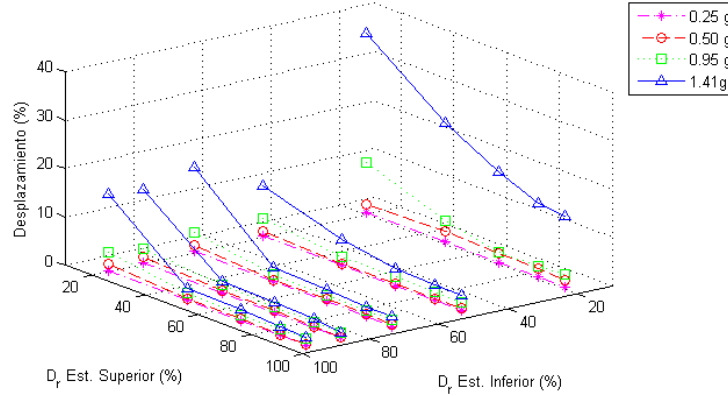
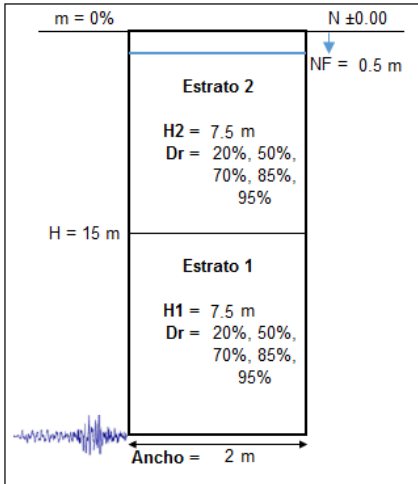


Scenario 6

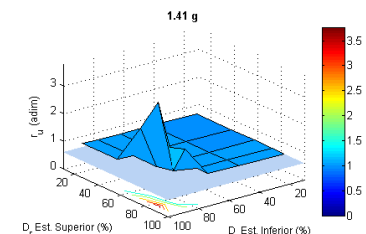
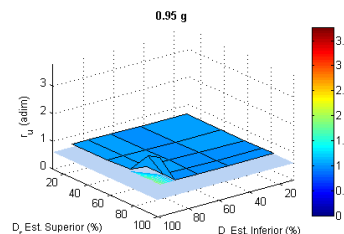
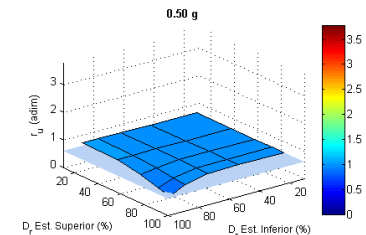
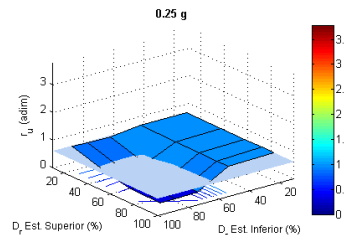
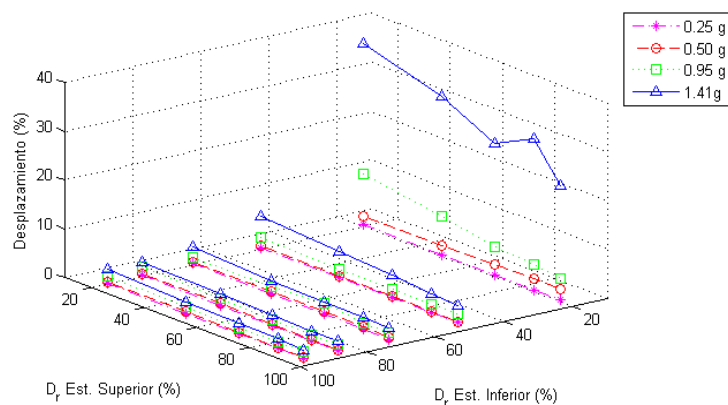
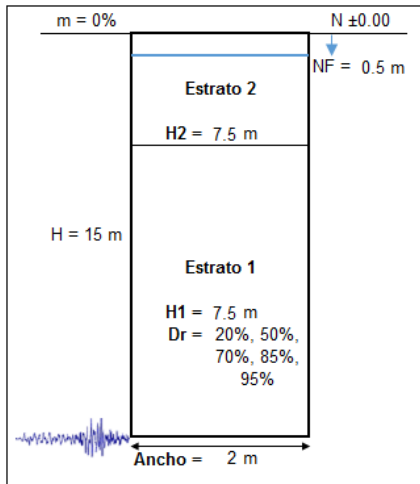


Numerical modeling

Scenario 2



Scenario 9

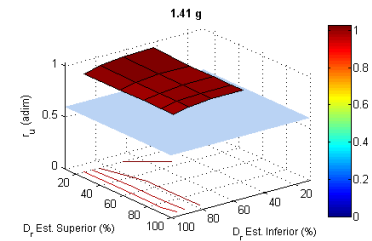
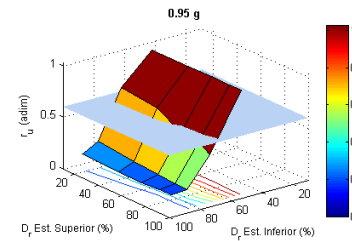
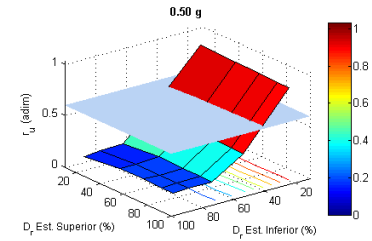
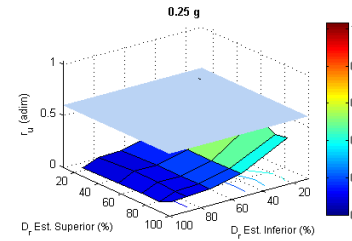
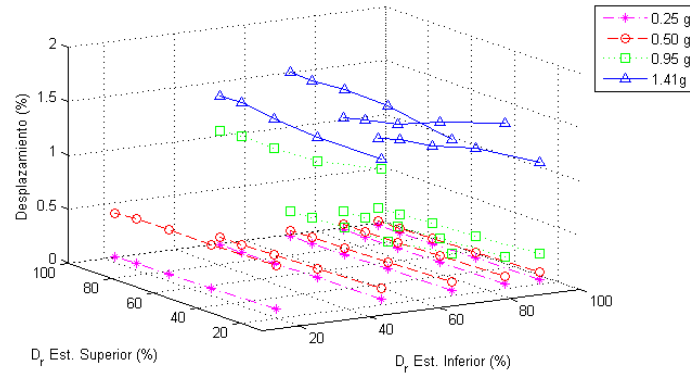
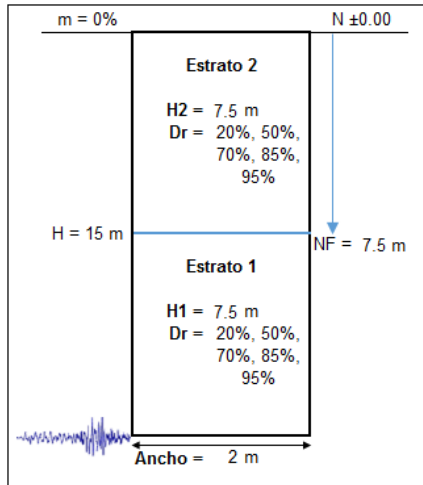




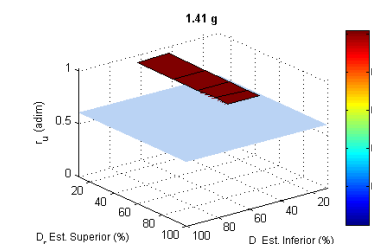
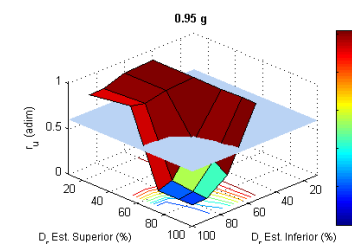
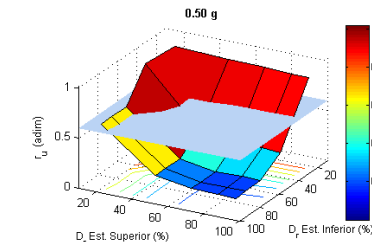
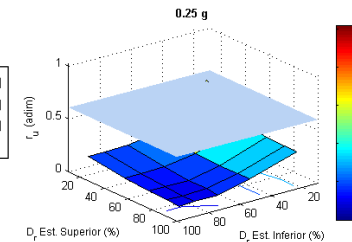
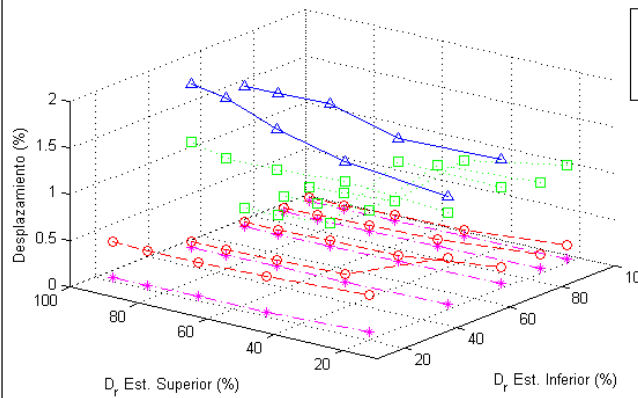
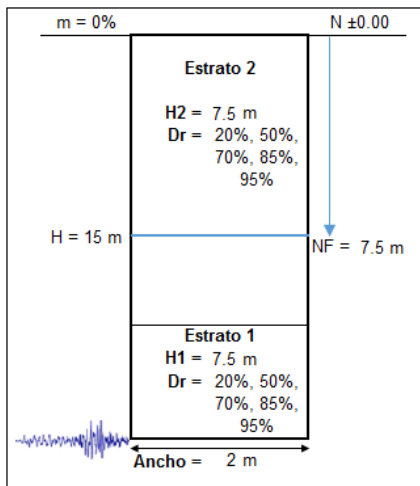
Influence of Soil Stratification and Water Level

Numerical modeling

Scenario 4



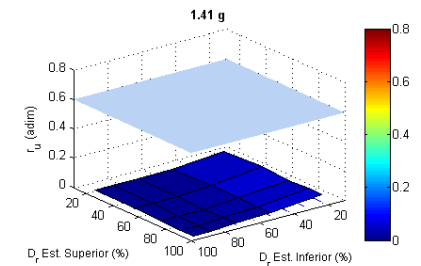
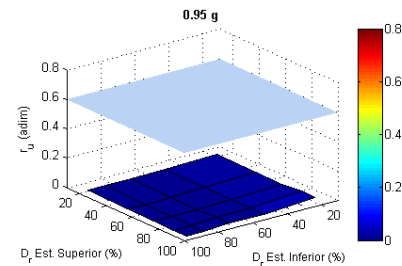
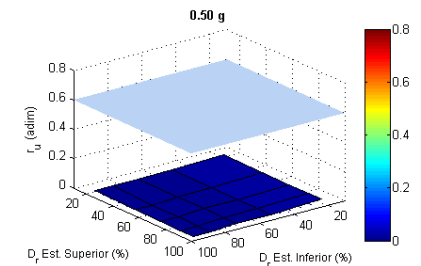
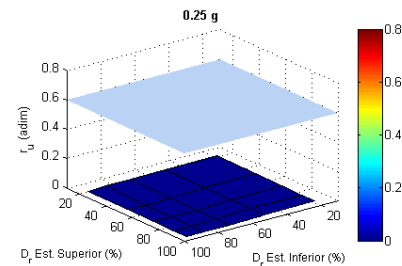
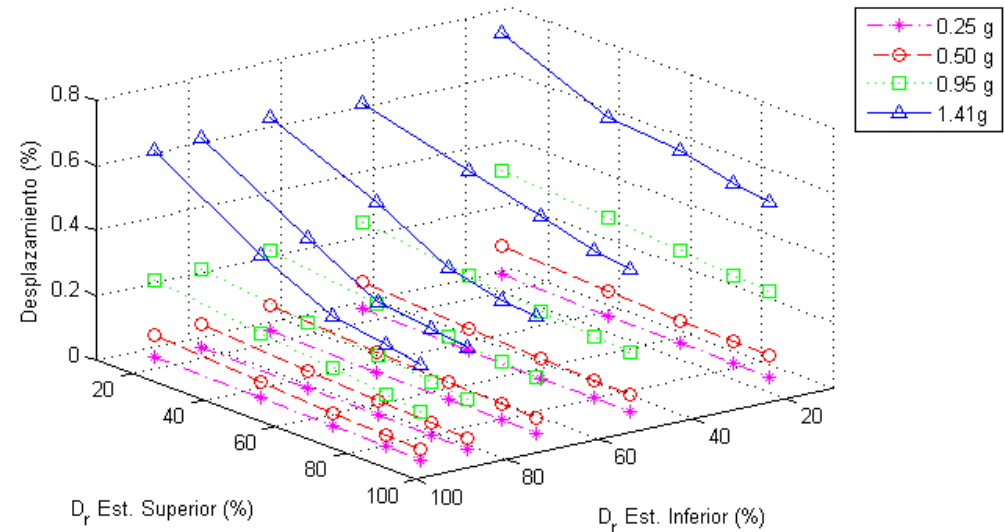
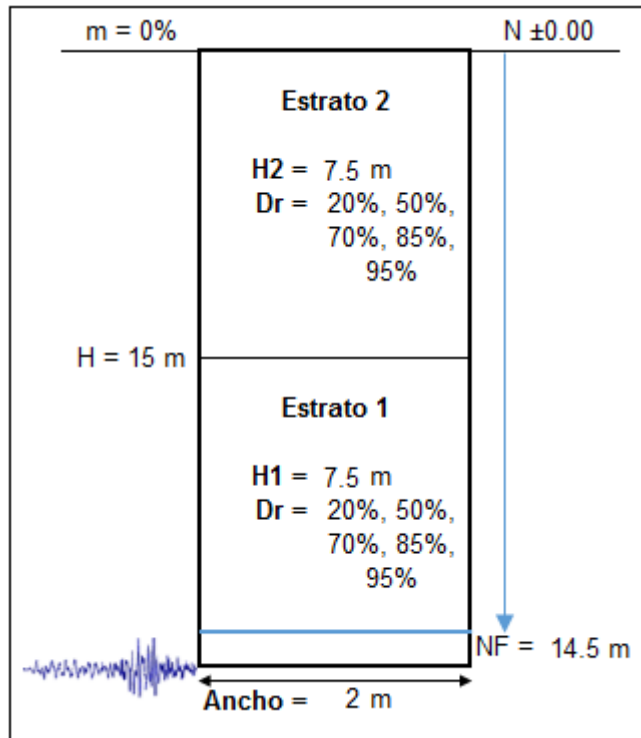
Scenario 8



Influence of the Water Level

Numerical modeling

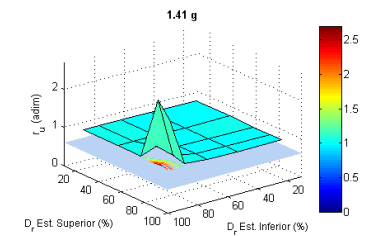
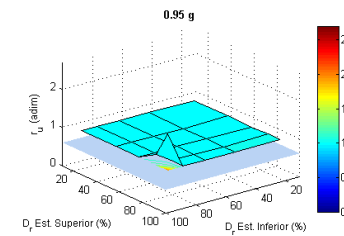
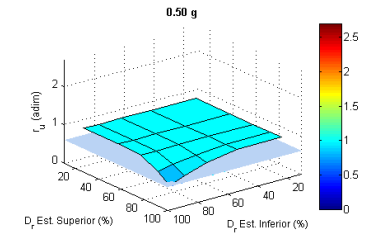
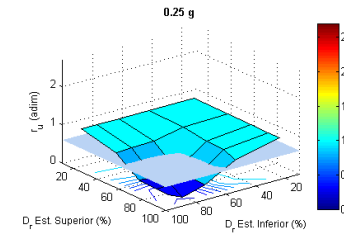
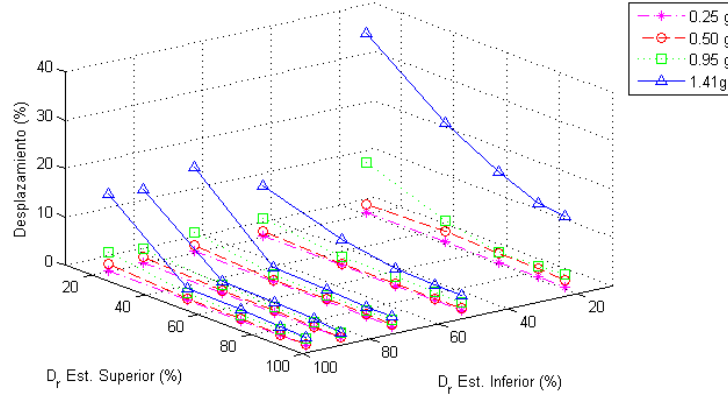
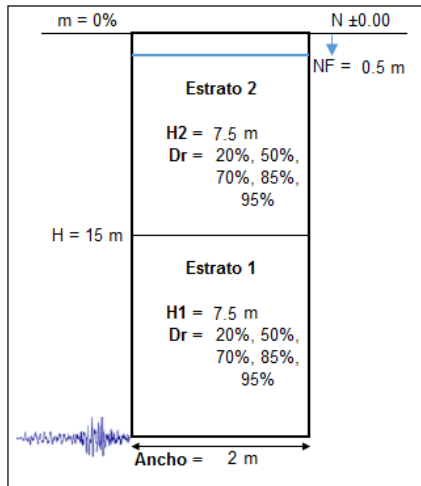
Scenario 4



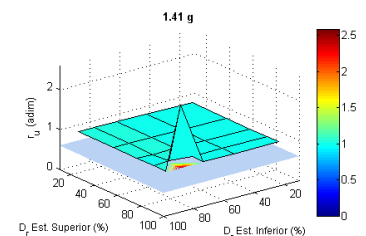
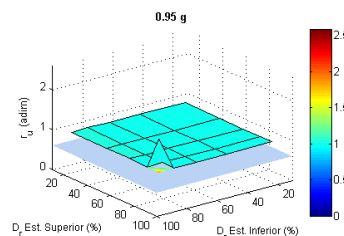
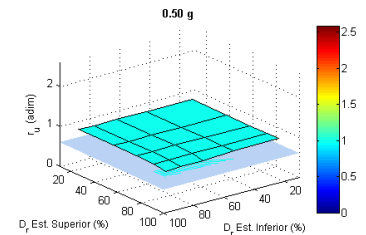
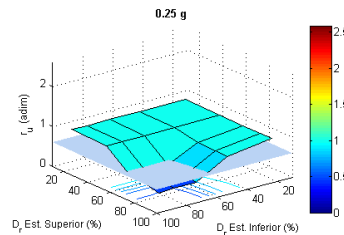
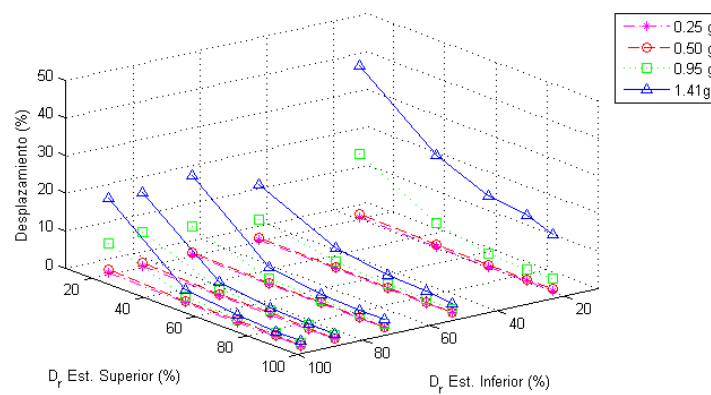
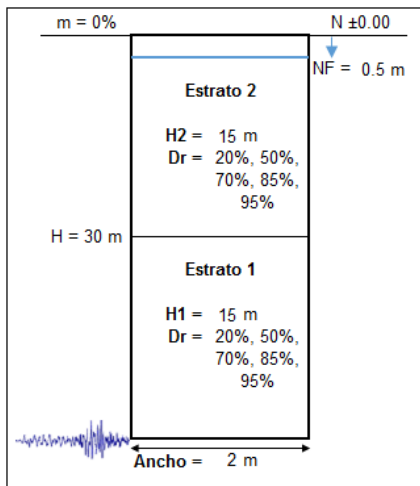
Influence of Height of the floor column

Numerical modeling

Scenario 2

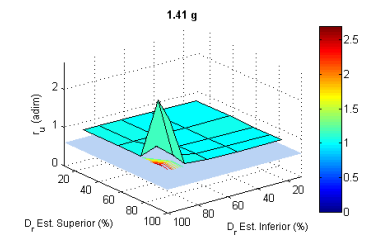
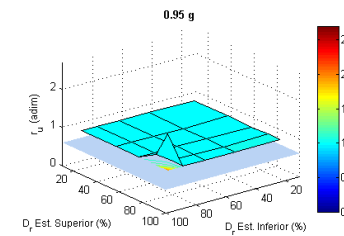
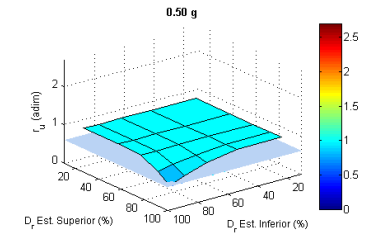
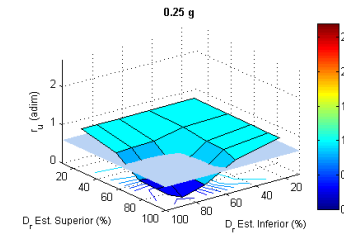
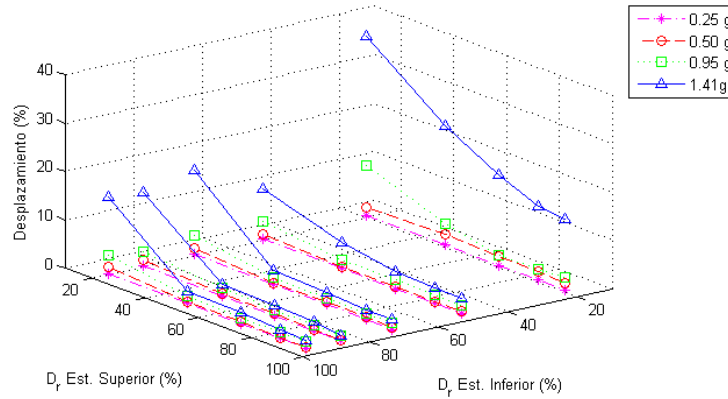
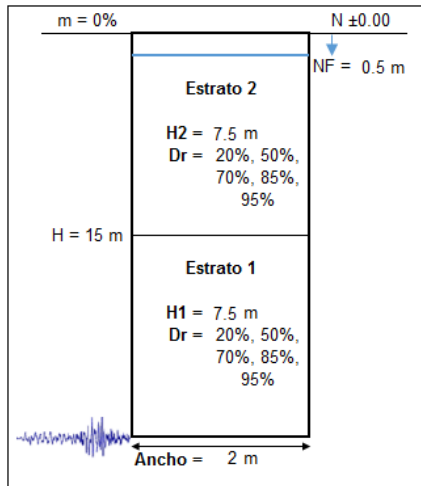


Scenario 10

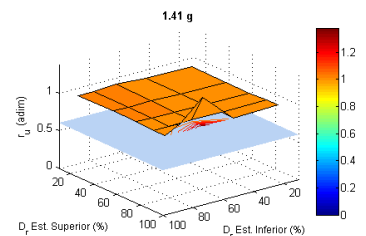
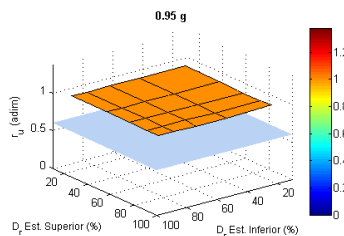
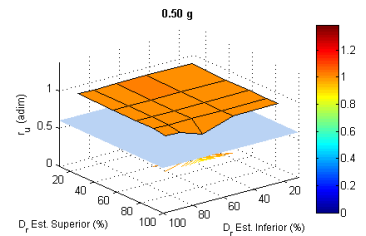
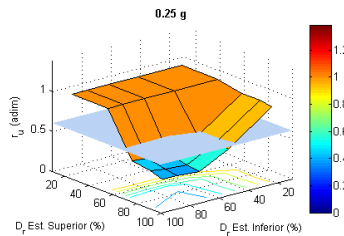
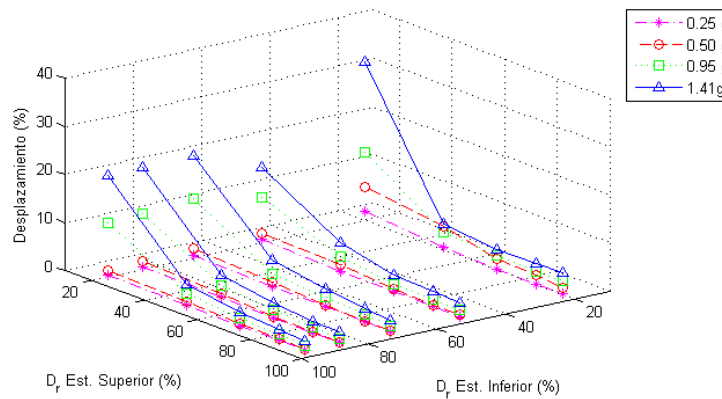
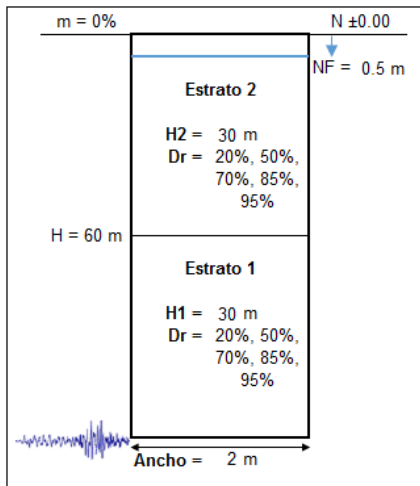


Numerical modeling

Scenario 2



Scenario 13



CONCLUSIONS

Conclusions

- ❑ A computer tool has been created to analyze the phenomenon of soil liquefaction using OpenSees software, which simulates a soil column in two dimensions with the application of a seismic load acting on a base.
- ❑ The use of the computer tool for the modeling of different scenarios has allowed to verify certain theoretical foundations on the occurrence of soil liquefaction.
- ❑ It was found that the location of the water table is the variable with the highest incidence in the activation of liquefaction.
- ❑ It could be inferred that the phenomenon of liquefaction in a soil column is activated for certain intensities of earthquake, in dense soils ($D_r > 85\%$) intensities close to 1g are required, and in shallow soils ($D_r < 50$), intensities from 0.25 g they produce liquefaction.

Conclusions

- ❑ The height of the soil column has no significant influence on the manifestation of the soil liquefaction phenomenon when the conditions of seismic load intensities, relative densities and soil stratification are the same.
- ❑ The potential of liquefaction of soils did not show significant variations due to stratigraphy as long as the height of the soil column and the water table remain constant.

Gracias



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